# REQUEST FOR HEARING <br> PLANNING COMIMISSION: OAK HILL, TENNESSEE 

Date Submitted: 08/10/22
PC Meeting Date: 10/04/22
The undersigned hereby requests consideration for a decision of the Planning
Commission of Oak Hill, Tennessee, wherein Baird Graham
Owner/Developer/Architect/Engineer
of the property located at: 1167 Travelers Ridge Drive Nashville, TN 37220
Lot Number(s): 39 Subdivision: Inns of Granny White
The property is in Zoning District $\qquad$ , in accordance with plans, application, and all data filed with the City of Oak Hill.

| Radnor Lake Natural Area Impact Zone | or |  |
| :--- | :---: | :--- |
| Steep Slope | N | or |
| Plat/Subdivision | Y | or |

Project Explanation: Building New Single family Home
Planning Commission Meeting Date: 10/04/22

Baird Graham
Applicant Name (Name)


Parcel No. $\qquad$

921 Robertson Academy Road Nashulle, TN
37220 baird@bgc-construction.com Applicant Email Address

City of Oak Hill (Signature)

Fee Amount: $\$ 1250.00$

## SITE - GRADING PLANS NEW RESIDENCE 1167 TRAVELERS RIDGE DRIVE CITY of OAK HILL Nashville-Davidson County, Tennessee 34th Councilmanic District

| Sheet Index |  |
| :--- | :--- |
| 1 | TITLE |
| 2 | EXISTING CONDITIONS |
| 3 | SITE PLAN |
| 4 | GRADING \& UTILITY PLAN |
| 5 | RAIN GARDEN PLAN |
| 6 | EROSION CONTROL PLAN |
| 7 | DISTURBED AREA |
| 8 | TREE PLAN |
| 9 | DETAILS |

Civil Engineer
Tony Snyder
Snyder Engineering pllc
228 Spence Lane
Nashville, Tennessee 37210 615-383-1699
tonysnyder@comcast.net
Surveyor
Campbell McRae \& Associates Inc. P.O.Box 41153

Nashville, Tennessee 37204
Phone 615-298-2424 cmas@att.net
Architect
Zinc Architecture
5820 Fredricksburg Drive
Nashville, Tennessee 37215
www. zincarch.com

UTILITIES

> | Electric Service |
| :--- |
| Nashville Electric Service |
| 1214 Church St. |
| Nashville, Tennessee 37246 |
| Joe Vallelev 615-747-3261 |

Water Service Metro Water Servic 1600 2nd Ave. N Nashville, Tennessee 37208 Christian Thompson -615-862-7229

Sewer Service
Metro Water Servic
1600 2nd Ave. N
Nashville, Tennessee 37208 615-862-4598

## Gas Service

Piedmont Natural Gas
800-752-7504




## $20 \%$ OF 8,967 SF $=1,793$ S

1,793 SF / 50 SF PER TREE $=36$ TREES
BY INSPECTION THERE ARE MORE THAN 36 TREES REMAINING that Are at Least 6" DBh.

AREA TO BE TREATED $=8,967-1,793=7,174$ SF
DEPTH OF SOIL MEDIA $=36$ INCHES
REQUIRED RAIN GARDEN SURFACE AREA $=361$ S
RAIN GARDEN SURFACE AREA PROVIDED $=390$ SF








## bgc construction travelers ridge

1167 TRAVELERS RIDGE NASHVILLE, TN 37220 MAY 24,2021

bgc
trave
bgc
travelers ridge 1167 travelers ridge
nashvile, tn 37220

HEIGHT ZONE CALCULATION / PLAN
SCALE: $1 / 322^{\prime \prime}=1$ '-O"
ISSUE FOR PERMIT 05.24.21





LOMER LEVEL Shet No: AlOO






| WINDOW SCHEDULE |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| tag | WIDTH | HEIGHT | operation |  | EXTERIOR | MODEL No. | QuAntity | COMMENTS |
| A | 7-O" | $9{ }^{\text {a }}$ | FIXED | ND | CLAD | - | - | - |
| B | 3-0" | $2^{2}-0 /$ | FIXED | ND | CLAD | - | - |  |
| ${ }^{\circ}$ | $3-\mathrm{O}^{\prime \prime}$ | 5-0" | CASEMENT | ND | CLAD | - | - | EGRESS |
| D | 91-6" | $9-6$ | FIXED | ND | CLAD | - | - |  |
| E | 3 -0" | 91-6" | FIXED | ND | CLAD | - | - | - |
| F | ${ }^{1-4 "}$ | $4{ }^{4}-0^{\prime \prime}$ | CASEMENT | ND | Clad | - | - | $-$ |
| 6 | 3 -- ${ }^{\prime \prime}$ | 10'-0" | FIXED | ND | CLAD | - | - | SIDELIGHT |
| + | 3-0" | 3'-0" | ANWIN | ND | CLAD | - | - | $-$ |
| ј |  |  | FIXED | ND WD | CLAD | - | - | $-$ |
| $\llcorner$ | 5-8" | T'-O" | FIXED | ND | CLAD | - | - | - |
| M | 2'-0" | $3-0 \mid$ | ANVING | ND | CLAD | - | - | - |
| $\cdots$ | 2'-0" | 7-O" | FIXED | ND | CLAD | - | - |  |
| P | 3 3-0" | 3 3-0" | FIXED | ND | Clad | - | - |  |
| ${ }^{\circ}$ | 3-0" | $6^{\prime \prime-O}$ | CASEMENT | ND | CLAD | - | - | EGRESS |
| R | 5-8" | $3{ }^{3}-0^{\prime \prime}$ | FIXED | ND | CLAD | - | - |  |
| $\stackrel{5}{5}$ | $\frac{5-8 " 1}{2-0^{\prime \prime}}$ |  | FIXED | ND | CLAD | - | - | - |
| U | ${ }^{2}-\mathrm{O}^{\prime \prime}$ | 6'-O" | CASEMENT | ND | CLAD | - | - | - |
| $\checkmark$ | 4-4" | $6^{\prime}-0^{\prime \prime}$ | CASEMENT | ND | CLAD | - | - | - |
| ${ }^{\sim}$ | 1-6" | $8{ }^{\text {- }}$ - ${ }^{\text {a }}$ | CASEMENT | ND | CLAD | - | - | - |
| $\times$ | 4-8" | 8 8-0" | F\|XED | ND | CLAD | - | - | - |
| $r$ | 4-0" | 9 9-O" | F\|XED | ND | CLAD | - | - | - |
| z | 8'-0" | 8'-0" | FIXED | ND | CLAD | - | - | - |
| ${ }_{\text {A }}{ }_{\text {E }}$ | 4-O" | $\frac{2^{\prime \prime-0 "}}{0^{\prime \prime}}$ | F\|XED | ND | CLAD | - | - | - |
| ${ }_{\text {co }}$ | $10^{1}-0^{\prime \prime}$ | ${ }^{10} 0^{\prime \prime} 0^{\prime \prime}$ | $\stackrel{\text { FIXED }}{ }$ | ND | ${ }_{\text {CLLAD }}$ | - | - | - |
| Do | $3{ }^{3}-6{ }^{\prime \prime}$ | $9{ }^{10} 0^{\prime \prime}$ | F\|XED | ND | CLAD | - | - | - |
| EE | 3-4" | $22^{1-6 "}$ | FIXED | ND | CLAD | - | - | - |
| fF | 3-4" | 5-6" | CASEMENT | ND | CLAD | - | - | EGRESS |
| 66 | 2-O" | $8{ }^{\text {8-O" }}$ | CASEMENT | ND | CLAD | - | - |  |
| +4 | $6^{6} \cdot 100^{\prime \prime}$ | $6^{6}-0^{\prime \prime}$ | F\|XED | ND | CLAD | - | - | - |
| JJ | $\frac{2-8 " 1}{2-8 \prime \prime}$ | 3'-0" | AMNNG | $\stackrel{N D}{\text { ND }}$ | CLAD | - | - | - |
| L | $2^{1}-6^{\prime \prime}$ | 3-O" | AXNIS | ND | CLAD | - | - | - |
| MM | 2'-6" | $3-0 \mid$ | FIXED | ND | CLAD | - | - | - |
| N | 3'-8" | 2'-6" | AWNIN | ND | CLAD | - | - | - |
| Pp |  |  | $\stackrel{\text { F\|XED }}{\text { F\|XED }}$ | ND ND | CLAD | - | - |  |
| RR | 3'-0" | $5^{51-6 "}$ | CASEMENT | MD | CLAD | - | - | EGRESS |




DOOR \&NNDOM
SCHEDULES
shetrivo
A700


September 14, 2022

## Mr. Baird Graham

BGC Construction
2510 Franklin Pike
Nashville, Tennessee 37204

## RE: Site Plan Review

Lot 39, Inns of Granny White Subdivision
City of Oak Hill, Tennessee
AG \& E File Number: 2022-025

Dear Mr. Graham:

As requested, we have reviewed the Site Grading Plan and the retaining wall design for the above referenced building lot.

The Site Grading Plan was prepared by Snyder Engineering, dated September 13, 2022. This plan complies with the Geotechnical Engineering Study we prepared on May 27, 2022.

We have also prepared the structural plans and details for the retaining walls. These details are enclosed with this letter.

Thank you for the opportunity to be of service to you in this matter. If you should have any questions concerning this or any other matter, please feel free to contact us at your convenience.

Sincerely yours,
AMERICAN GEOTECHNICAL \& ENVIRONMENTAL, INC.


Robert T. Stickney, P.E.
President
Enclosure


Notes: All concrete shall be Class "A" Concrete with a minimum strenath of 3,500 psi.
Reinforcing steel to conform to ASTM A 615, Grade 60
Construction joints will be provided at a maximum spacing of 30 feet.
Minimum splice lenath for \# 4 bars $=20$ inches
Provide Min, 3 inches of Cover for the reinforcina steel

| PROJECT N0. |  |
| :--- | :--- |
| 2022-025 |  |
| SCALE |  |
| FIGURE NO. 1 |  |

Project Name/Number : lot 39 travel
Title 4 Ft Wall with 3:1 Backslope
Page: 1 Dsgnr:

Date: 14 SEP 2022
Description....

This Wall in File: c:luserslbobstldocuments\retainpro project files\lot 39 travelers ridge.rpx

| RetainPro (c) 1987-2019, Build 11.20.03.31 License: KW-06061779, License To : American Geotechnical | Cantilevered Retaining Wall <br> \& Environmental, Inc. | Code: IBC 2012,ACI 318-11,ACI 530-11 |
| :---: | :---: | :---: |
| Criteria |  |  |
| Retained Height | $=6.00 \mathrm{ft}$ |  |
| Wall height above soil | 0.50 ft |  |
| Slope Behind Wall | $=3.00$ |  |
| Height of Soil over Toe | 24.00 in |  |
| Water height over heel | $=0.0 \mathrm{ft}$ | $\because:$ |

## Load Factors

| Building Code | IBC 2012,ACI |
| :--- | ---: |
| Dead Load | 1.200 |
| Live Load | 1.600 |
| Earth, H | 1.600 |
| Wind, W | 1.000 |
| Seismic, E | 1.000 |

Soil Data and Lateral Earth Pressure

| Allow Soil Bearing | $=$ | $3,000.0 \mathrm{psf}$ | Soil Density, Heel <br> Equivalent Fluid Pressure Method |  |  |
| :--- | :--- | :--- | :--- | :--- | :---: |
| Soil Density, Toe |  |  |  |  |  |$\quad$| 120.00 pcf |
| :---: |
| Active Heel Pressure |

## Surcharge Loads

| Surcharge Over Heel <br> Used To Resist Sliding \& Overturning | 0.0 psf | Surcharge Over Toe <br> Used for Sliding \& Overturning | 0.0 |
| :--- | :---: | :---: | :---: |

Axial Load Applied to Stem

| Axial Dead Load | $=$ | 0.0 lbs | Axial Load Eccentricity | $=$ |
| :--- | :--- | :--- | :--- | :--- |
| Axial Live Load | $=$ | 0.0 lbs |  | 0.0 in |

## Lateral Load Applied to Stem

| Lateral Load | $=$ | $0.0 \# / \mathrm{ft}$ |
| :--- | :--- | :--- |
| $\ldots$. Height to Top | $=$ | 0.00 ft |
| $\ldots$ Height to Bottom | $=$ | 0.00 ft |
| Load Type | $=$ | Wind (W) |
| (Service Level) |  |  |
| Wind on Exposed Stem |  |  |

Wind on Exposed Stem $\quad=\quad 0.0 \mathrm{psf}$
(Service Level)

## Adjacent Footing Load

| Adjacent Footing Load | $=$ | 0.0 lbs | Footing Type | Line Load |
| :--- | :--- | :--- | :--- | :--- |
| Footing Width | $=$ | 0.00 ft | Base Above/Below Soil |  |
| Eccentricity | $=$ | 0.00 in | at Back of Wall | $=$ |
| Wall to Ftg CL Dist | $=$ | 0.00 ft | Poisson's Ratio | $=$ | Title $\quad 4 \mathrm{Ft}$ Wall with 3:1 Backslope

This Wall in File: c:lusers\bobstldocuments\retainpro project files\lot 39 travelers ridge.rpx


## Sliding

## Resisting Forces

| Vertical Forces | $\underline{r}$ Force |
| :--- | ---: |
| Soil Over Heel (above water table, if any) | $1,020.0$ |
| lbs |  |
| Soil Over Heel (below water table, if any) | 0.0 |
| Water Over Heel | 0.0 |
| Buoyant Force | 0.0 |
| Sloped Soil Over Heel | 40.1 |
| Surcharge Over Heel | 0.0 |
| Adjacent Footing Load | 0.0 |
| Axial Dead Load on Stem | 0.0 |
| Axial Live Load on Stem * | 320.0 |
| Soil Over Toe | 0.0 |
| Surcharge Over Toe | 650.0 |
| Stem Weight(s) | 0.0 |
| Earth @ Stem Transitions | 512.5 |
| Footing Weight | 0.0 |
| Key Weight | 0.0 |
| Vert. Component ** | $2,542.6$ lbs |
| Total Vertical Loads |  |

## Sliding Forces

| Lateral Forces | Force |
| :--- | ---: |
| Heel Active Pressure (above water table, if any) | 893.3 lbs |
| Heel Active Pressure (below water table, if any) | 0.0 |
| Hydrostatic Force | 0.0 |
| * Heel Active Pressure | 893.3 |
| Surcharge over Heel | 0.0 |
| Adjacent Footing | 0.0 |
| Surcharge Over Toe | 0.0 |
| Load @ Stem Above Soil | 0.0 |
| Added Lateral Load | 0.0 |
| Seismic Load | 0.0 |
| Seismic-Self-weight | 0.0 |
| Lateral on Key | 0.0 |
| Totals = |  |
|  | 893.3 lbs |
| *Includes water table effect |  |
|  |  |

* Axial live load NOT included in total displayed, or used for overturning or sliding resistance, but is included for soil pressure calculations.


## Sliding Calcs

| Lateral Sliding Force | $=$ |  | 893.3 lbs |
| :--- | :--- | :---: | :---: |
| less $100 \%$ Passive Force | $=$ | - | $1,000.0 \mathrm{lbs}$ |
| less 100\% Friction Force | $=$ | - | $1,017.1 \mathrm{lbs}$ |
| Added Force Req'd | $=$ |  | 0.0 lbs OK |
| ...for 1.5 Stability | $=$ | 0.0 lbs OK |  |

This Wall in File: c:lusers\bobstldocuments\retainpro project files\lot 39 travelers ridge.rpx
RetainPro(c) 1987-2019, Build 11.20.03.31 Cantilevered Retaining Wall
License: KW-06061779
License To : American Geotechnical \& Environmental, Inc.

License To : American Geotechnical \& Environmental, Inc.

## Overturning

## Resisting Moments

| Resisting Moments | Force | Distance |  | Moment |
| :---: | :---: | :---: | :---: | :---: |
| Soil Over Heel (above water table, if any) | 1,020.0 | lbs | 2.71 ft | 2,762.5ft-\# |
| Soil Over Heel (below water table, if any) | 0.0 |  |  |  |
| Water Table | 0.0 |  |  |  |
| Soil Over Heel | 1,020.0 |  | 2.71 | 2,762.5 |
| Sloped Soil Over Heel | 40.1 |  | 2.94 | 118.2 |
| Surcharge Over Heel | 0.0 |  |  |  |
| Adjacent Footing Load | 0.0 |  |  |  |
| Axial Dead Load on Stem | 0.0 |  |  |  |
| Axial Live Load on Stem * | 0.0 |  |  |  |
| Soil Over Toe | 320.0 |  | 0.67 | 213.3 |
| Surcharge Over Toe | 0.0 |  |  |  |
| Stem Weight(s) | 650.0 |  | 1.67 | 1,083.3 |
| Earth @ Stem Transitions | 0.0 |  |  |  |
| Footing Weight | 512.5 |  | 1.71 | 875.5 |
| Key Weight | 0.0 |  | 1.33 |  |
| Vert. Component | 0.0 |  |  |  |
| Total Vertical Loads | 2,542.6 | lbs |  |  |
| Resis | oment |  |  | 5,052.9 ft-\# |
| Eccen |  |  |  | -6.5 in |

* Axial live load NOT included in total displayed, or used for overturning or sliding resistance, but is included for soil pressure calculations.


## Overturning

## Overturning Moments

Overturning Moments
Heel Active Pressure (above water table, if any)
Heel Active Pressure (below water table, if any)


This Wall in File: c:lusers\bobstldocuments\retainpro project files\lot 39 travelers ridge.rpx
RetainPro (c) 1987-2019, Build 11.20.03.31 Cantilevered Retaining Wall
License: KW-06061779,
License To : American Geotechnical \& Environmental, Inc.

License To : American Geotechnical \& Environmental, Inc.

## Stem Design Summary

|  |  | Bottom |
| :---: | :---: | :---: |
|  |  | Stem OK |
| Design Height Above Ftg | $\mathrm{ft}=$ | 0.00 |
| Wall Material Above "Ht" | = | Concrete |
| Design Method | = | LRFD |
| Thickness | = | 8.00 |
| Rebar Size | $=$ | \# 4 |
| Rebar Spacing | = | 9.00 |
| Rebar Placed at | = | Edge |
| Design Data |  |  |
| $\mathrm{fb} / \mathrm{FB}+\mathrm{fa} / \mathrm{Fa}$ | $=$ | 0.258 |
| Total Force @ Section Service Level | $\mathrm{lbs}=$ |  |
| Strength Level | $\mathrm{lbs}=$ | 921.6 |
| Moment....Actual Service Level | ft -\# = |  |
| Strength Leve | ft -\# = | 1,843.2 |
| Moment.....Allowable | = | 7,122.4 |
| Shear.....Actual |  |  |
| Service Level | psi = |  |
| Strength Leve | psi = | 12.3 |
| Shear.....Allowable | psi $=$ | 75.0 |
| Anet | in2 = |  |
| Rebar Depth 'd' | in $=$ | 6.25 |
| Masonry Data |  |  |
| f'm | psi $=$ |  |
| Fs | psi $=$ |  |
| Solid Grouting | = |  |
| Modular Ratio ' n ' | = |  |
| Wall Weight | psf = | 100.0 |
| Short Term Factor | = |  |
| Equiv. Solid Thick. | = |  |
| Masonry Block Type | = | Medium Weight |
| Masonry Design Method | $=$ | ASD |
| Concrete Data f'c Fy |  |  |
|  | psi $=$ | 2,500.0 |
|  | psi $=$ | 60,000.0 |

This Wall in File: c:luserslbobstldocuments\retainpro project filesllot 39 travelers ridge.rpx

| RetainPro (c) 1987-2019, Build 11.20.03.31 License $:$ KW-06061779 License To : American Geotechnical \& | Cantile <br> Environmental, Inc. | etaining Wal | Code: IBC 2012,ACI 318-11,ACI 530-11 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Concrete Stem Rebar Area Details |  |  |  |  |  |
| Bottom Stem | Vertical Reinforcing | Horizontal Reinforcing |  |  |  |
| As (based on applied moment) : | 0.0691 in2/ft | Min Stem T\&S Reinf Area 1.248 in |  |  |  |
| (4/3) * As : | $0.0921 \mathrm{in} 2 / \mathrm{ft}$ |  |  |  |  |
| 200bd/fy : 200(12)(6.25)/60000 : | $0.25 \mathrm{in} 2 / \mathrm{ft}$ | Min Stem T\&S Reinf Area per ft of stem Height : 0.192 in2/ft |  |  |  |
| $0.0018 \mathrm{bh}: 0.0018(12)(8)$ : | $0.1728 \text { in2/ft }$ | Horizontal Reinforcing Options : |  |  |  |
|  | =========== | One layer of : | Two layers of : |  |  |
| Required Area : | 0.1728 in2/ft | \#4@ 12.50 in | \#4@ 25.00 in |  |  |
| Provided Area : | $0.2667 \mathrm{in} 2 / \mathrm{ft}$ | \#5@19.38 in | \#5@38.75 in |  |  |
| Maximum Area : | $0.8467 \mathrm{in} 2 / \mathrm{ft}$ | \#6@ 27.50 in | \#6@ 55.00 in |  |  |
| Footing Data |  |  |  |  |  |
| Toe Width | 1.33 ft | f'c |  | = | 3,000 ps |
| Heel Width | 2.08 | Fy |  | = | 60,000 ps |
| Total Footing Width | 3.42 ft | Footing Concr | crete Density | = | 150.00 pc |
| Footing Thickness | $=\quad 12.00$ in | Min. As \% |  | = | 0.0018 |
| Key Width | 0.00 in | Rebar Cover | @ Top | = | 2.00 in |
| Key Depth | 0.00 in |  | @ Bottom | = | 3.00 in |
| Key Distance from Toe | 1.33 ft |  |  |  |  |

## Footing Design Results

|  | Toe |  | Heel |  |
| :---: | :---: | :---: | :---: | :---: |
| Factored Pressure | = | 2,031 | 52 | psf |
| Mu' : Upward | = | 18,923 | 327 | ft -\# |
| Mu' : Downward | = | 4,992 | 1,093 | ft -\# |
| Mu: Design | = | 1,161 | -48 | ft -\# |
| Actual 1-Way Shear | = | 7.88 | 7.65 | psi |
| Allow 1-Way Shear | = | 82.16 | 82.16 | psi |
| Toe Reinforcing |  | 9.00 in |  |  |
| Heel Reinforcing |  | 9.00 in |  |  |
| Key Reinforcing |  | pec'd |  |  |

Other Acceptable Sizes \& Spacings
Toe: \#4@ 9.25 in, \#5@ 14.35 in, \#6@ 20.37 in, \#7@ 27.77 in, \#8@ 36.57 in, \#9@ 46 Heel: \#4@9.25 in, \#5@ 14.35 in , \#6@ 20.37 in , \#7@ 27.77 in, \#8@ 36.57 in , \#9@ 46
Key: No key defined

| Min footing T\&S reinf Area | 0.89 | in2 |
| :--- | :--- | :--- |
| Min footing T\&S reinf Area per fo | 0.26 | in2 $/ \mathrm{ft}$ |

If one layer of horizontal bars:
\#4@ 9.26 in
\#5@ 14.35 in
\#6@ 20.37 in
Footing Torsion, Tu
Footing Allow. Torsion, phi Tu

If two layers of horizontal bars:
\#4@18.52 in
\#5@ 28.70 in
\#6@ 40.74 in

If torsion exceeds allowable, provide supplemental design for footing torsion.

This Wall in File: c:lusers\bobstldocuments\retainpro project files\lot 39 travelers ridge.rpx
RetainPro(c) 1987-2019, Build 11.20.03.31 Cantilevered Retaining Wall
License: KW-06061779
License To : American Geotechnical \& Environmental, Inc.

## Tilt

## Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

| Soil Spring Reaction Modulus | 250.0 pci |
| :--- | :--- | :--- |
| Horizontal Defl @ Top of Wall (approximate only) | 0.077 in |

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe,
because the wall would then tend to rotate into the retained soil.

Project Name/Number : lot 39 travel
Title 6 Ft Wall with 3:1 Backslope
Page: 1 Dsgnr:

Date: 14 SEP 2022
Description....

This Wall in File: c:luserslbobstldocumentslretainpro project files\lot 39 travelers ridge.rpx

| RetainPro (c) 1987-2019, Build 11.20.03.31 |  | Cantilevered Retaining Wall | Code: IBC 2012,ACI 318-11,ACI 530-11 |
| :--- | :--- | :--- | :--- |
| License: KW-06061779 <br> License To $:$ American Geotechnical \& Environmental, Inc. |  |  |  |
| Criteria |  |  |  |
| Retained Height | $=$ | 8.00 ft |  |
| Wall height above soil | $=$ | 0.50 ft |  |
| Slope Behind Wall | 3.00 |  |  |
| Height of Soil over Toe | $=$ | 24.00 in |  |
| Water height over heel | $=$ | 0.0 ft |  |

## Load Factors

| Building Code | IBC 2012,ACI |
| :--- | ---: |
| Dead Load | 1.200 |
| Live Load | 1.600 |
| Earth, H | 1.600 |
| Wind, W | 1.000 |
| Seismic, E | 1.000 |

Soil Data and Lateral Earth Pressure

| Allow Soil Bearing | $=$ | $3,000.0 \mathrm{psf}$ | Soil Density, Heel <br> Equivalent Fluid Pressure Method |  |  |
| :--- | :--- | :--- | :--- | :--- | :---: |
| Soil Density, Toe |  |  |  |  |  |$\quad$| 120.00 pcf |
| :---: |
| Active Heel Pressure |

## Surcharge Loads

| Surcharge Over Heel <br> Used To Resist Sliding \& Overturning | 0.0 psf | Surcharge Over Toe <br> Used for Sliding \& Overturning | 0.0 |
| :--- | :---: | :---: | :---: |

Axial Load Applied to Stem

| Axial Dead Load | $=$ | 0.0 lbs | Axial Load Eccentricity | $=$ |
| :--- | :--- | :--- | :--- | :--- |
| Axial Live Load | $=$ | 0.0 lbs |  | 0.0 in |

## Lateral Load Applied to Stem

| Lateral Load | $=$ | $0.0 \# / \mathrm{ft}$ |
| :--- | :--- | :--- |
| $\ldots$ Height to Top | $=$ | 0.00 ft |
| $\ldots$. Height to Bottom | $=$ | 0.00 ft |
| Load Type | $=$ | Wind (W) |
| Wind on Exposed Stem |  |  |

Wind on Exposed Stem $\quad=\quad 0.0 \mathrm{psf}$
(Service Level)

## Adjacent Footing Load

| Adjacent Footing Load | $=$ | 0.0 lbs | Footing Type | Line Load |
| :--- | :--- | :--- | :--- | :--- |
| Footing Width | $=$ | 0.00 ft | Base Above/Below Soil |  |
| Eccentricity | $=$ | 0.00 in | at Back of Wall | $=$ |
| Wall to Ftg CL Dist | $=$ | 0.00 ft | Poisson's Ratio | $=$ |
| 0.0 | 0.0 ft |  |  |  | Title 6 Ft Wall with 3:1 Backslope

This Wall in File: c:lusers\bobstldocuments\retainpro project files\lot 39 travelers ridge.rpx

| RetainPro (c) 1987-2019, Build 11.20.03.31 <br> License : KW-06061779 <br> License To : American Geotechnical \& Environmental, Inc. |  |  |
| :---: | :---: | :---: |
| Wall Design Summary |  |  |
| Stability Ratios |  |  |
| Overturning | = | 2.32 OK |
| Sliding | = | 1.71 OK |
| Soil Bearing |  |  |
| Total Bearing Load | = | 3,866 lbs |
| ...resultant ecc. | = | 8.22 in |
| Soil Pressure @ Toe | = | 1,690 psf OK |
| Soil Pressure @ Heel | = | 61 psf OK |
| AllowableSoil Pressure Less Than Allowable |  |  |
|  |  |  |
| ACI Factored @ Toe | = | 2,366 psf |
| ACI Factored @ Heel | = | 85 psf |
| Footing Shear @ Toe | = | 16.7 psi OK |
| Footing Shear @ Heel | = | 13.4 psi OK |
| Allowable | = | 82.2 psi |

## Sliding

## Resisting Forces

## Sliding Forces

| Vertical Forces | Force |
| :--- | ---: |
| Soil Over Heel (above water table, if any) | $1,840.0$ |
| lbs |  |
| Soil Over Heel (below water table, if any) | 0.0 |
| Water Over Heel | 0.0 |
| Buoyant Force | 0.0 |
| Sloped Soil Over Heel | 73.5 |
| Surcharge Over Heel | 0.0 |
| Adjacent Footing Load | 0.0 |
| Axial Dead Load on Stem | 0.0 |
| Axial Live Load on Stem * Omit |  |
| Soil Over Toe | 440.0 |
| Surcharge Over Toe | 0.0 |
| Stem Weight(s) | 850.0 |
| Earth @ Stem Transitions | 0.0 |
| Footing Weight | 662.5 |
| Key Weight | 0.0 |
| Vert. Component ** | 0.0 |
| Total Vertical Loads | $3,866.0$ |


| Lateral Forces | Force |
| :--- | ---: |
| Heel Active Pressure (above water table, if any) | $1,486.5 \mathrm{lbs}$ |
| Heel Active Pressure (below water table, if any) | 0.0 |
| Hydrostatic Force | 0.0 |
| * Heel Active Pressure | $1,486.5$ |
| Surcharge over Heel | 0.0 |
| Adjacent Footing | 0.0 |
| Surcharge Over Toe | 0.0 |
| Load @ Stem Above Soil | 0.0 |
| Added Lateral Load | 0.0 |
| Seismic Load | 0.0 |
| Seismic-Self-weight | 0.0 |
| Lateral on Key | 0.0 |
| Totals = |  |
|  | $1,486.5 \mathrm{lbs}$ |
| *Includes water table effect |  |
|  |  |

* Axial live load NOT included in total displayed, or used for overturning or sliding resistance, but is included for soil pressure calculations.


## Sliding Calcs

| Lateral Sliding Force | $=$ |  | $1,486.5 \mathrm{lbs}$ |
| :--- | :--- | :--- | :--- |
| less $100 \%$ Passive Force | $=$ | - | $1,000.0 \mathrm{lbs}$ |
| less $100 \%$ Friction Force | $=$ | - | $1,546.4 \mathrm{lbs}$ |
| Added Force Req'd | $=$ |  | 0.0 lbs OK |
| ...for 1.5 Stability | $=$ | 0.0 lbs OK |  |

This Wall in File: c:lusers\bobstldocuments\retainpro project files\lot 39 travelers ridge.rpx
RetainPro(c) 1987-2019, Build 11.20.03.31 Cantilevered Retaining Wall
License: KW-06061779
License To : American Geotechnical \& Environmental, Inc.

License To : American Geotechnical \& Environmental, Inc.

## Overturning

## Resisting Moments

| Resisting Moments | Force | Distance |  | Moment |
| :---: | :---: | :---: | :---: | :---: |
| Soil Over Heel (above water table, if any) | 1,840.0 | lbs | 3.46 ft | 6,363.3ft-\# |
| Soil Over Heel (below water table, if any) | 0.0 |  |  |  |
| Water Table | 0.0 |  |  |  |
| Soil Over Heel | 1,840.0 |  | 3.46 | 6,363.3 |
| Sloped Soil Over Heel | 73.5 |  | 3.78 | 277.6 |
| Surcharge Over Heel | 0.0 |  |  |  |
| Adjacent Footing Load | 0.0 |  |  |  |
| Axial Dead Load on Stem | 0.0 |  |  |  |
| Axial Live Load on Stem * | 0.0 |  |  |  |
| Soil Over Toe | 440.0 |  | 0.92 | 403.3 |
| Surcharge Over Toe | 0.0 |  |  |  |
| Stem Weight(s) | 850.0 |  | 2.17 | 1,841.7 |
| Earth @ Stem Transitions | 0.0 |  |  |  |
| Footing Weight | 662.5 |  | 2.21 | 1,463.0 |
| Key Weight | 0.0 |  | 1.83 |  |
| Vert. Component | 0.0 |  |  |  |
| Total Vertical Loads | 3,866.0 | lbs |  |  |
| Resis | ment |  |  | 10,348.9 ft-\# |
| Eccen |  |  |  | -8.2 in |

* Axial live load NOT included in total displayed, or used for overturning or sliding resistance, but is included for soil pressure calculations.


## Overturning

## Overturning Moments

Overturning Moments
Heel Active Pressure (above water table, if any)
Heel Active Pressure (below water table, if any)

| Force | Distance | Moment |
| :---: | :---: | :---: |
| 1,486.5 lbs | 3.00 ft | 4,776.2 ft-\# |
| 0.0 0.0 |  |  |
| 0.0 |  |  |
| 0.0 |  |  |
| 0.0 |  |  |
| 0.0 |  |  |
| 0.0 |  |  |
| 0.0 |  |  |
| 0.0 |  |  |
| 0.0 |  |  |
| 1,486.5 lbs |  |  |
| Overturning | Moment | 4,459.6 ft-\# |

This Wall in File: c:lusers\bobstldocuments\retainpro project files\lot 39 travelers ridge.rpx
RetainPro (c) 1987-2019, Build 11.20.03.31 Cantilevered Retaining Wall
License: KW-06061779,
License To : American Geotechnical \& Environmental, Inc.

License To : American Geotechnical \& Environmental, Inc.

## Stem Design Summary

|  |  | Bottom |
| :---: | :---: | :---: |
|  |  | Stem OK |
| Design Height Above Ftg | $\mathrm{ft}=$ | 0.00 |
| Wall Material Above "Ht" | = | Concrete |
| Design Method | = | LRFD |
| Thickness | = | 8.00 |
| Rebar Size | $=$ | \# 4 |
| Rebar Spacing | = | 9.00 |
| Rebar Placed at | = | Edge |
| Design Data |  |  |
| $\mathrm{fb} / \mathrm{FB}+\mathrm{fa} / \mathrm{Fa}$ | $=$ | 0.613 |
| Total Force @ Section Service Level | $\mathrm{lbs}=$ |  |
| Strength Level | lbs = | 1,638.4 |
| Moment....Actual Service Level | ft -\# = |  |
| Strength Leve | ft -\# = | 4,369.1 |
| Moment.....Allowable | = | 7,122.4 |
| Shear.....Actual |  |  |
| Service Level | psi $=$ |  |
| Strength Leve | psi = | 21.8 |
| Shear.....Allowable | psi $=$ | 75.0 |
| Anet | in2 = |  |
| Rebar Depth 'd' | in $=$ | 6.25 |
| Masonry Data |  |  |
| f'm | psi $=$ |  |
| Fs | psi $=$ |  |
| Solid Grouting | = |  |
| Modular Ratio ' n ' | = |  |
| Wall Weight | psf = | 100.0 |
| Short Term Factor | = |  |
| Equiv. Solid Thick. | = |  |
| Masonry Block Type | = | Medium Weight |
| Masonry Design Method | $=$ | ASD |
| Concrete Data f'c Fy |  |  |
|  | psi $=$ | 2,500.0 |
|  | psi $=$ | 60,000.0 |

This Wall in File: c:luserslbobstldocuments\retainpro project files\lot 39 travelers ridge.rpx

| RetainPro (c) 1987-2019, Build 11.20.03.31 License: KW-06061779 License To : American Geotechnical \& | Cantile <br> Environmental, Inc. | etaining Wal | Code: IBC 2012,ACI 318-11,ACI 530-11 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Concrete Stem Rebar Area Details |  |  |  |  |  |
| Bottom Stem | Vertical Reinforcing | Horizontal Reinforcing |  |  |  |
| As (based on applied moment) : | $0.1637 \mathrm{in} 2 / \mathrm{ft}$ |  |  |  |  |
| (4/3) * As : | $0.2183 \mathrm{in} 2 / \mathrm{ft}$ | Min Stem T\&S Reinf Area 1.632 in2 |  |  |  |
| 200bd/fy : 200(12)(6.25)/60000 : | $0.25 \mathrm{in} 2 / \mathrm{ft}$ | Min Stem T\&S Reinf Area per ft of stem Height : 0.192 in2/ft |  |  |  |
| 0.0018bh : 0.0018(12)(8) : | 0.1728 in2/ft | Horizontal Reinforcing Options : |  |  |  |
|  | ============ | One layer of : | Two layers of : |  |  |
| Required Area : | $0.2183 \mathrm{in} 2 / \mathrm{ft}$ | \#4@ 12.50 in | \#4@ 25.00 in |  |  |
| Provided Area : | $0.2667 \mathrm{in} 2 / \mathrm{ft}$ | \#5@ 19.38 in | \#5@38.75 in |  |  |
| Maximum Area : | $0.8467 \mathrm{in} 2 / \mathrm{ft}$ | \#6@ 27.50 in | \#6@ 55.00 in |  |  |
| Footing Data |  |  |  |  |  |
| Toe Width | 1.83 ft | f'c |  | = | 3,000 ps |
| Heel Width | 2.58 | Fy |  | = | 60,000 ps |
| Total Footing Width | $=4.42 \mathrm{ft}$ | Footing Concrete Density |  | = | 150.00 po |
| Footing Thickness | 12.00 in | Min. As \% |  | = | 0.0018 |
| Key Width | 0.00 in | Rebar Cover | @ Top | = | 2.00 in |
| Key Depth | 0.00 in |  | @ Bottom | = | 3.00 in |
| Key Distance from Toe | 1.83 ft |  |  |  |  |

## Footing Design Results

|  |  | Toe | Heel |  |
| :---: | :---: | :---: | :---: | :---: |
| Factored Pressure | = | 2,366 | 85 | psf |
| Mu' : Upward | = | 41,345 | 762 | ft -\# |
| Mu' : Downward | = | 9,438 | 2,559 | ft -\# |
| Mu: Design | = | 2,659 | -308 | ft -\# |
| Actual 1-Way Shear | = | 16.67 | 13.42 | psi |
| Allow 1-Way Shear | = | 82.16 | 82.16 | psi |
| Toe Reinforcing | $=$ | 9.00 in |  |  |
| Heel Reinforcing |  | 9.00 in |  |  |
| Key Reinforcing | = | Spec'd |  |  |

Other Acceptable Sizes \& Spacings
Toe: \#4@ 9.25 in, \#5@ 14.35 in , \#6@ 20.37 in , \#7@ 27.77 in, \#8@ 36.57 in , \#9@ 46 Heel: \#4@9.25 in, \#5@ 14.35 in , \#6@ 20.37 in , \#7@ 27.77 in, \#8@ 36.57 in , \#9@ 46
Key: No key defined


If torsion exceeds allowable, provide supplemental design for footing torsion.

This Wall in File: c:lusers\bobstldocuments\retainpro project files\lot 39 travelers ridge.rpx
RetainPro(c) 1987-2019, Build 11.20.03.31 Cantilevered Retaining Wall
License: KW-06061779
License To : American Geotechnical \& Environmental, Inc.
Tilt

## Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

| Soil Spring Reaction Modulus | 250.0 pci |
| :--- | :--- | :--- |
| Horizontal Defl @ Top of Wall (approximate only) | 0.090 in |

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe,
because the wall would then tend to rotate into the retained soil.

## Kimley») Horn

## MEMORANDUM

| To: | Mr. Stephen Snow |
| :--- | :--- |
| From: | Zac Dufour, P.E. |
|  | Kimley-Horn and Associates, Inc. |
| Date: | September 14, 2022 |
| Subject: | PC Case 22-27, 1167 Travelers Ridge |

We have completed our review of the revised Steep Slope site plan for the proposed new home located at 1167 Travelers Ridge. Please see below for engineering comments.

## Comments

1. Removing trees greater than $8^{\prime \prime}$ in the front yard will require BZA approval.
a. Variance will be needed for removal of trees within the front yard. JARIANCE APPL. SUBuMTED
2. Update Sheet 2 to show proper bulk standards. See markup.
a. Revised
3. Provide tree survey of all trees $3^{\prime \prime}$ in diameter and greater.
a. Provided.
4. Verify front setback based on average of 4 closest lots.
a. Verified and provided.
5. Rear setback is $60^{\prime}$.
a. Revised.
6. Bold numbers in center of lot on Sheet 3 are not correct. Update to current bulk standards.
a. Revised,
7. Remove plat reference to setback lines.
a. Removed.
8. Driveway must be 5 ' off of property line. Add dimensions.
a. Dimensions provided.
9. Provide street cut areas around utility connections for sanitary and water, reference metro street cut detail and provide detail on plans.
a. Provided and noted. Show sawcut area and limits of disturbance for water line connection. PROVIDED, SEE SHEET H
10. Provide more spot grades in driveway and around outside of house.
a. Outstanding comment. Need more spot grades around the house and driveway. PROVIDED
11. Label proposed contours.

SHEE4
a. Provided.
12. Provide more grades on wall in rear of house.
a. Provided.
13. Retaining wall engineering drawings and calculations are required for walls over $4^{\prime}$ in height.
a. Wall drawings have been provided. Structural calculations must be provided as well. TO BE PRONIDED BY OWNER
14. Provide proposed grading in front of house to match architectural plans - there appears to be about a 20' difference between existing grade and FFE.
FRONT RIGHT CORNER OF HOMSE EXIST. के PROPOSED GRADE IS 778.00. MAIN FLOOR FFE IS 791.30. DIFFERENEE 1513.30 FEET. IHS THE GROUND LINE SHOWN ON THE ARCH. PLANS IS HRIGHTLY OFF.

## Kimley »>Horn

a. Architectural plans have been provided. Additional proposed ground grades will help understand the amount of exposed foundation around the exterior of the house. THE EXPOSED FOUNDATION IS 13.3 FEET MAXIMUM.
15. Show cleanouts on the MFD.
a. MFD has been replaced with a rain garden. Show cleanouts or structures along the 6 " pipe run leading to the rain garden.
16. Is the pipe in the patio intended to be a trench drain? If so label and provide a detail. Need to make sure this water is captured and routed to the MFD.
a. This appears to just be a pipe to connect downspout to rain garden. YES
17. MFD is sized for 5275 but the added impervious area is 7995 . MFD must be oversized to account for impervious bypass.
a. Rain Garden is sized for full impervious area.
18. Is there a way to grade the driveway and maybe install trench drains to capture more of the driveway area and route it to the MFD?
a. Show drainage area to rain garden. Is some of the driveway going towards the rain garden. Rain garden must be sized for the drainage area not just for the added impervious area. DONE
19. Add a trench drain at the bottom of the driveway to capture water and route to ditch. Add detail for the trench drain.
a. This has not been added. $2 \%$ cross slope is less than the longitudinal slope of the driveway so not all of the runoff will sheet flow into the roadside ditch. TRENCH DRA
20. Provide outlet protection for the daylight end of the MFD pipe.
a. Show outlet protection for rain garden. How will rain garden outlet - must provide underdrains and show outlet for underdrains. DONE -
b. Provide site specific detail for the rain garden with elevations of each layer and materials. DONE
21. Show downspout locations and routing to MFDs.
a. Rrovided.
22. Note trees that are to be removed on the plans.
a. Provided.
23. Tabulate the trees that are to be removed in a table and provide the total caliper inches that are to be removed.
a. Provided.
24. Need to draw in the drip line of all trees. Any proposed improvements or grading that gets into these drip lines needs to have the tree removed and be accounted for in the removal.
a. Provided.
25. Need to show tree protection fencing around all trees that are to be saved. Provide detail of tree protection fencing.
a. Provided. Show detail for tree protection fence. DONE
26. Need to provide tree canopy coverage exhibit showing the exiting canopy on the lot and the \% of lot canopy coverage. Show proposed canopy that is to be removed. Include table from the tree ordinance.
a. Provided
27. Provide architectural floor plans and elevations.
a. Provided.
28. Provide an elevation exhibit showing conformance to the recently updated building height regulations. Show actual height to top of roof line. Show the calculated dimensions of the Zone 1, Zone 2 and Zone 3 areas on a plan view exhibit. Show Zone 3 on the side elevation.
a. Not provided.
29. Provide Gross Floor Area Ratio on plans.
a. Not provided. Provide
30. Provide a statement from geotechnical engineer stating that they have reviewed the current site and grading plans and they comply with the geotechnical recommendations.
a. Provided
31. Provide a statement from structural engineer stating that they have reviewed the current site, grading plans and geotechnical report and the structural drawings are consistent with all other plans.
a. Provided.
32. Geotechnical engineer shall be on site during construction to observe conditions and report on the conditions with respect to the initial study, boring data, lab testing and provide any updated recommendations based on any deviations. Geotechnical engineer shall provide a certification letter upon completion of construction prior to the issuance of a certification of occupancy. The certification letter shall speak to the construction methods, geotechnical recommendations that were followed during construction, geotechnical engineer observations during construction and any deviations from the original recommendations that were made.
a. Noted.
33. Add steep slope geotechnical requirements per the Steep Slope Ordinance Section 14-238. Add note, "Geotechnical Engineer shall be on site during construction to monitor construction. Engineer shall submit a geotechnical certification letter certifying the stability of the slope and the structure to the City of Oak Hill upon completion of construction and prior to the issuance of a certificate of occupancy."
a. Provided.
34. Additional comments may be forthcoming from the Geotechnical engineering review.

You must provide a comment response letter for all of the above comments to be considered for the October Planning Commission.

All revised plans, calculations, and any other supporting documentation along with the full comment response letter must be submitted in email by September 20, 2022.

## c: File

## Kimley»"Horn

## MEMORANDUM

To: Mr. Stephen Snow<br>From: Zac Dufour, P.E.<br>Kimley-Horn and Associates, Inc.<br>Date: August 30, 2022<br>Subject: PC Case 22-27, 1167 Travelers Ridge

We have completed our review of the Steep Slope site plan for the proposed new home located at 1167 Travelers Ridge. Please see below for engineering comments.

## Comments

1. Removing trees greater than 8 " in the front yard will require BZA approval.
2. Update Sheet 2 to show proper bulk standards. See markup.
3. Provide tree survey of all trees $3^{\prime \prime}$ in diameter and greater.
4. Verify front setback based on average of 4 closest lots.
5. Rear setback is 60 '.
6. Bold numbers in center of lot on Sheet 3 are not correct. Update to current bulk standards.
7. Remove plat reference to setback lines.
8. Driveway must be 5' off of property line. Add dimensions.
9. Provide street cut areas around utility connections for sanitary and water, reference metro street cut detail and provide detail on plans.
10. Provide more spot grades in driveway and around outside of house.
11. Label proposed contours.
12. Provide more grades on wall in rear of house.
13. Retaining wall engineering drawings and calculations are required for walls over 4 ' in height.
14. Provide proposed grading in front of house to match architectural plans - there appears to be about a 20' difference between existing grade and FFE.
15. Show cleanouts on the MFD.
16. Is the pipe in the patio intended to be a trench drain? If so label and provide a detail. Need to make sure this water is captured and routed to the MFD.
17. MFD is sized for 5275 but the added impervious area is 7995 . MFD must be oversized to account for impervious bypass.
18. Is there a way to grade the driveway and maybe install trench drains to capture more of the driveway area and route it to the MFD?
19. Add a trench drain at the bottom of the driveway to capture water and route to ditch. Add detail for the trench drain.
20. Provide outlet protection for the daylight end of the MFD pipe.
21. Show downspout locations and routing to MFDs.
22. Note trees that are to be removed on the plans.
23. Tabulate the trees that are to be removed in a table and provide the total caliper inches that are to be removed.

## Kimley»"Horn

24. Need to draw in the drip line of all trees. Any proposed improvements or grading that gets into these drip lines needs to have the tree removed and be accounted for in the removal.
25. Need to show tree protection fencing around all trees that are to be saved. Provide detail of tree protection fencing.
26. Need to provide tree canopy coverage exhibit showing the exiting canopy on the lot and the \% of lot canopy coverage. Show proposed canopy that is to be removed. Include table from the tree ordinance.
27. Provide architectural floor plans and elevations.
28. Provide an elevation exhibit showing conformance to the recently updated building height regulations. Show actual height to top of roof line. Show the calculated dimensions of the Zone 1, Zone 2 and Zone 3 areas on a plan view exhibit. Show Zone 3 on the side elevation.
29. Provide Gross Floor Area Ratio on plans.
30. Provide a statement from geotechnical engineer stating that they have reviewed the current site and grading plans and they comply with the geotechnical recommendations.
31. Provide a statement from structural engineer stating that they have reviewed the current site, grading plans and geotechnical report and the structural drawings are consistent with all other plans.
32. Geotechnical engineer shall be on site during construction to observe conditions and report on the conditions with respect to the initial study, boring data, lab testing and provide any updated recommendations based on any deviations. Geotechnical engineer shall provide a certification letter upon completion of construction prior to the issuance of a certification of occupancy. The certification letter shall speak to the construction methods, geotechnical recommendations that were followed during construction, geotechnical engineer observations during construction and any deviations from the original recommendations that were made.
33. Add steep slope geotechnical requirements per the Steep Slope Ordinance Section 14-238. Add note, "Geotechnical Engineer shall be on site during construction to monitor construction. Engineer shall submit a geotechnical certification letter certifying the stability of the slope and the structure to the City of Oak Hill upon completion of construction and prior to the issuance of a certificate of occupancy."
34. Additional comments may be forthcoming from the Geotechnical engineering review.

Please provide revised plans, calculations, any other supporting documentation and a comment response letter by September 13, 2022 via email.
c: File

October 2, 2022

Mr. Stephen Snow
Code Enforcement Officer
City of Oak Hill

Re: Report of Geotechnical Review Services
Lot 39 - Inns of Granny White Subdivision
1167 Travelers Ridge
Oak Hill, Tennessee

## Mr. Snow:

At your request, Geo-Technology Associates, Inc. (GTA) has reviewed geotechnical information provided for the development of 1167 Travelers Ridge. The planned improvement is to include a residential construction and its associated driveway. The purpose of our review is to evaluate the information provided in the geotechnical report as it relates to the geotechnical aspects of design and the appropriate standard of care.

## PROJECT UNDERSTANDING

The following information was provided for review :

- Report titled, "Geotechnical Engineering Study, Lot 39, Inns of Granny White Subdivision, City of Oak Hill, Tennessee," prepared by American Geotechnical and Environmental, Inc. dated May 7, 2022
- Site - Grading Plans prepared by Snyder Engineering, PLLC dated August 30, 2022

The planned improvement will include a one- to two-story, residential structure. Based on the drawings provided, the first level will have a finished floor elevation of 791.3; with a garage level established at 782.0.

According to the site plan, the ground surface elevation across the proposed building areas varies from about elevation 790 within the western limits of the proposed structure to elevation 774 in the eastern limits of the proposed structure.

## REVIEW AND CONCLUSIONS

American Geotechnical and Environmental, Inc (AG\&E) observed the excavation of five test pits to explore the subsurface conditions at the site. Each of the test pits encountered topsoil underlain by colluvial soils extending to depths of 5.6 feet to 9.0 feet below existing site grades. The colluvial soils consist of two layers. The upper layer is a brown cherty silty clay that is underlain by brown to yellowish brown silty clays that contain some chert fragments and some slickenside faces at depths of 3.7 to 8.2 feet. Slickenside faces are secondary structures in the soil that have been smoothed, or polished, by movement of the soil. The lower stratum of colluvial soil extends to depths of 5.6 to 9.0 feet; beneath the colluvial soil is residual silty clays.

Bedrock was not explored, however, based on the review of the available geologic maps, "Geologic Map of the Oak Hill Quadrangle, Tennessee" (Tennessee Division of Geology 1972). The site is underlain buy limestone of the Leipers and Catheys Formations, which consist of thin- to mediumbedded shaly limestone.

Due to the presence of the colluvial soils, AG\&E performed numerous slope stability analysis to assess the stability of the subgrade as a result of the proposed construction. The slope stability analysis yielded unfavorable results based on standard shallow foundation construction. Accordingly, the analysis we performed based on a revised foundation construction. Specifically, AG\&E recommends that the proposed foundation excavations extend to a depth necessary to completely penetrate the colluvial soil and bear at least 2 feet into the underlying residual soils.

Accordingly, the foundation installation may require excavations on the order of 8 feet to 11 feet to completely penetrate the colluvial soils and expose the residual soils.

We recommend that AG\&E be on site during foundation excavations to confirm that foundation excavations completely penetrate the colluvial soil and extend at least 2 feet into the underlying residual soil.

In our opinion, the AG\&E, Inc.'s geotechnical report fulfills the geotechnical requirements for development.

We trust that this letter meets your immediate needs. If you require additional information, please let us know.

## Sincerely,

Geo-Technology Associates, Inc.


Daniel D. Terranova, PE

