

Permit #: 61
Permit Date: 05/02/23
Permit Type: Planning Commission
Case Number: PC 23-22
PC Meeting Date: j. 1st Tuesday of November
BZA Meeting Date:
Assigned Meeting Date: 11/07/2023
Special Meeting Date:
Applicant Is: Contractor
Applicant Name: ELY Mandle
Applicant Address: PO BOX 647
Applicant City, State, ZIP: Oxford MS 38655
Applicant Phone Number: 731-426-3964
Applicant Email: ely@ascentnashville.com
Description: Proposed structurally supported parking pad constructed in the steep slope.
Project Cost: 2000
Square Feet: 0
Lot Area: 0
Lot Coverage: 0
Heat/cooled area: 0
Proposed Height(ft.): 0
#of stories: 0
Lot Depth/Width Ratio:
Avg. front setback of adjacent homes:
Zoning District: Zone C
Radnor Lake Impact Zone: Yes
Steep Slope: Yes
Plat/Subdivison: No
Status: Open
Assigned To: Steve Mallory

Property

Parcel #	Address	Legal Description	Owner Name	Owner Phone	Zoning
14508003200	5031 LAKEVIEW DR	LOT 1 VILLA ESTATES	KE HOLDINGS LLC		

Fees

Fee	Description	Notes	Amount
Residential Radnor Lake Review Fee		(2 reviews)	\$1,000.00
Residential Steep Slope Review Fee		(2 reviews)	\$1,000.00
Total			\$2,000.00

Payments

Date	Paid By	Description	Payment Type	Accepted By	Amount
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05/02/2023

ELY Mandle

CK#8681

Steve Mallory

\$2,000.00

Outstanding Balance

\$0.00

Uploaded Files

Date	File Name
10/03/2023	17072219-Neighbor Letter residing at 1043 Ridgeview Dr..pdf
10/02/2023	17053590-PC 23-22 - 5031 Lakeview Dr Packet.pdf
10/02/2023	17053591-PC Submittal 5031 Lakeview.pdf
10/02/2023	17053592-Engineer Approval Letter.pdf
10/02/2023	17053587-5031 Photos.pdf
10/02/2023	17053588-5031 Resubmittal.2 82423.pdf













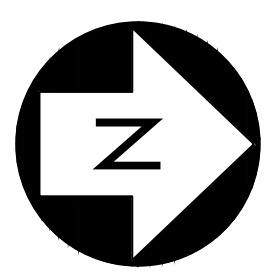
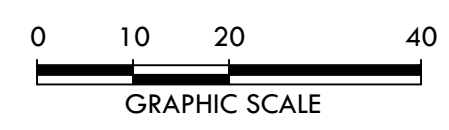






PROPOSED DEMOLITION LEGEND

- BUILDING DEMOLITION
- WOODEN DECK DEMOLITION
- CONCRETE PAVEMENT DEMOLITION
- EDGE OF PAVEMENT REMOVAL



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	1	07/26/2023

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C1.0
 DEMOLITION PLAN

Details Date: 05/19/2023 10:44 - 5031 Lakeview Drive Demolition Plan - 21014.dwg - 08/14/23



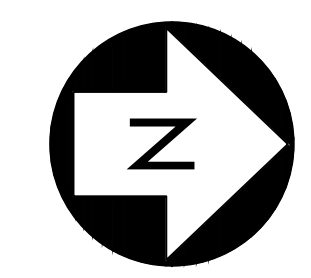
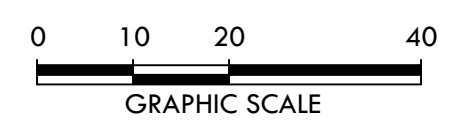
EROSION CONTROL PLAN KEYNOTES

CODE	DESCRIPTION	DETAIL LOCATION
E1	TEMPORARY SILT FENCE (2-ROWS TYPICAL)	C2.1 - 1
E2	TEMPORARY TREE PROTECTION	C2.1 - 2

PROPOSED EROSION & SEDIMENT CONTROL LEGEND

	TEMPORARY CONSTRUCTION ENTRANCE
	TEMPORARY TREE PROTECTION
	TEMPORARY SILT FENCE
	TEMPORARY STRAW BALE BARRIER
	TEMPORARY ROCK CHECK DAM
	LIMITS OF DISTURBANCE

- NOTES:
- TOTAL DISTURBED AREA IS 880 SF OR 0.02 ACRES
 - NO TREES ARE TO BE REMOVED
 - ALL EQUIPMENT UTILIZED WITHIN DISTURBED AREAS ARE REQUIRED TO LEAVE THE SITE VIA TRAILER. NO EQUIPMENT UTILIZED WITHIN DISTURBED AREAS SHALL BE DRIVEN WITHIN THE PUBLIC RIGHT OF WAY.
 - PERMANENT STABILIZATION WITH SOD ON ALL DISTURBED, STEEP SLOPED, GRASS AREAS SHALL BE PROVIDED.
 - PERMANENT STABILIZATION WITH PERENNIAL VEGETATION OR OTHER PERMANENTLY STABLE, NON-ERODING SURFACE SHALL REPLACE ANY TEMPORARY MEASURES AS SOON AS PRACTICABLE.
 - RIP-RAP PROTECTION TO BE INSTALLED UNDER THE PARKING DECK.



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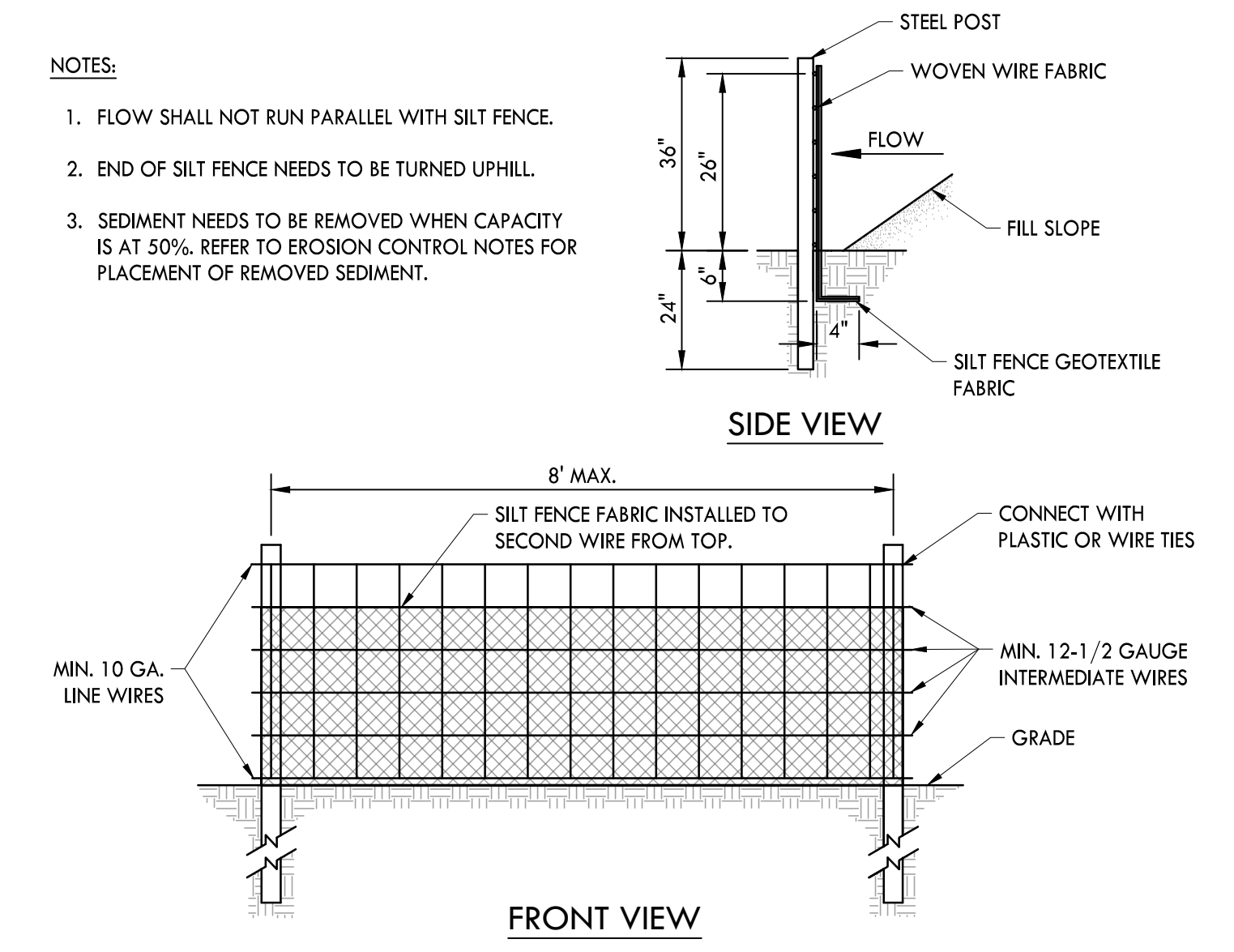
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	2	10/23/2023

10/23/23 21014

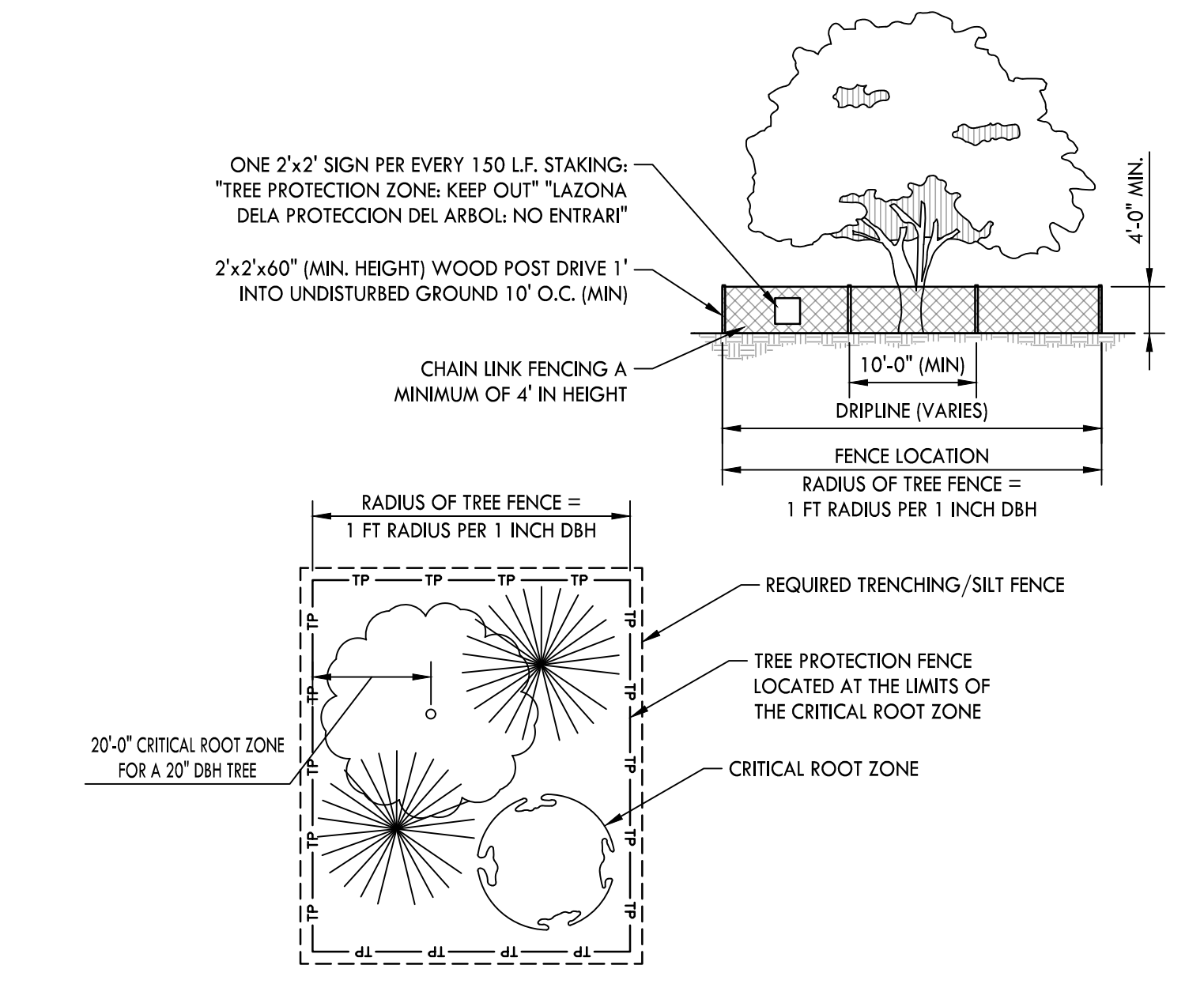
C2.0
 EROSION &
 SEDIMENT CONTROL
 PLAN

Drawing File Path: P:\2023\21014 - 5031 Lakeview Drive\Construction\21014_18_23_Plan\21014.dwg

NOTES:
 1. FLOW SHALL NOT RUN PARALLEL WITH SILT FENCE.
 2. END OF SILT FENCE NEEDS TO BE TURNED UPHILL.
 3. SEDIMENT NEEDS TO BE REMOVED WHEN CAPACITY IS AT 50%. REFER TO EROSION CONTROL NOTES FOR PLACEMENT OF REMOVED SEDIMENT.

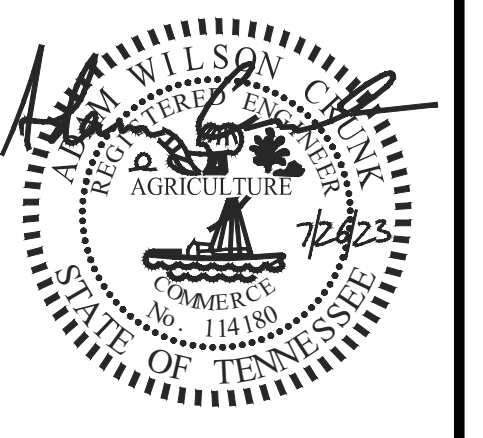


1
TEMPORARY SILT FENCE
 NOT TO SCALE



2
TEMPORARY TREE PROTECTION

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08/22/23 21014

C2.1
 EROSION AND
 SEDIMENT CONTROL
 DETAILS

EXISTING IMPERVIOUS AREA	PROPOSED IMPERVIOUS AREA
DRIVEWAYS/WALKS (INCLUDE DRIVE-APRONS PL) PATIOS/UNCOVERED PORCHES/DECKS RET. WALLS/MISC. HARDSCAPE/EQUIP.PADS/OTHER IA ROOFED AREA OF ALL BUILDINGS: 3049 SQ FT TOTAL EXISTING: 7659.95 SQ FT.	DRIVEWAYS/WALKS (INCLUDE DRIVE-APRONS PL) PATIOS/UNCOVERED PORCHES/DECKS RET. WALLS/MISC. HARDSCAPE/EQUIP.PADS/OTHER IA ROOFED AREA OF ALL BUILDINGS: 528 SQ FT. TOTAL PROPOSED: 8187.55 SQ FT. NET ADDITIONAL IA: 538 SQ FT. TOTAL EXIST/ NET IA, AS % OF TOTAL LOT SF: 6.89%

DOUGLAS, FUCHS AND WIFE
INSTRUMENT # 20080310-0028244
LOT 4 VILLA ESTATES

CURTIS & HEATHER THORNE
INSTRUMENT # 20110916-0072174
LOT 5 VILLA ESTATES

JOYCE & JOHN GRIFFEN
INSTRUMENT # 20111014-0080206
LOT 3 VILLA ESTATES

1/2" IRON
ROD SET
N:634427.83
E:1732074.51

P.O.B
1/2" IRON
ROD FOUND
N:634353.33
E:173224.52

A:64.77
RAD 248.78'
CB S 23°57'05" W
CL 64.39' (SURVEY)
A:64.17'
RAD 248.78'
CB S 20°08'31" W
CL 64.59' (DEED)

A:69.99'
RAD 800.10'
CB S 28°57'13" W
CL 69.97' (SURVEY)
A:69.99'
RAD 800.10'
CB S 25°06'39" W
CL 69.97' (DEED)

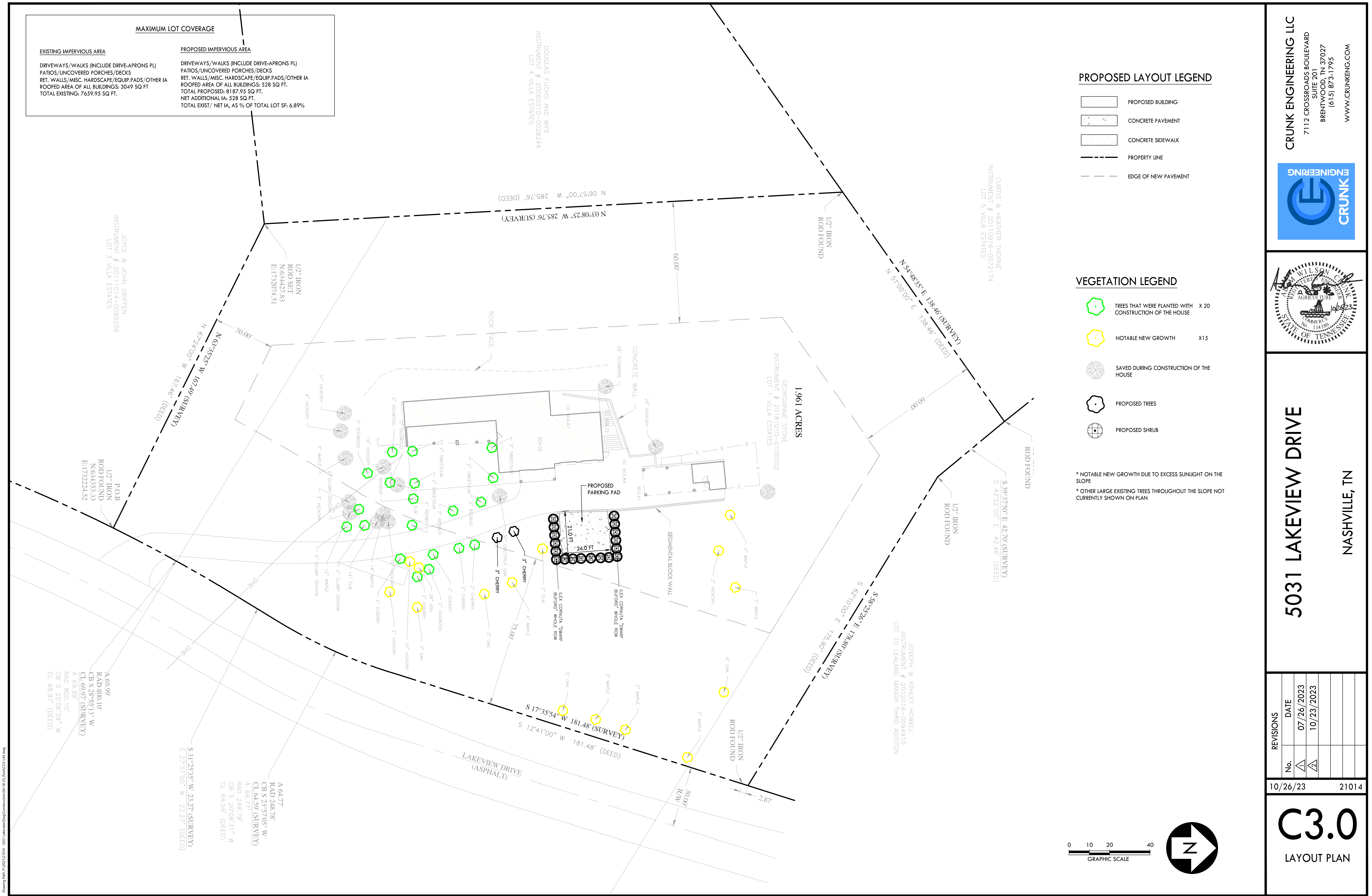
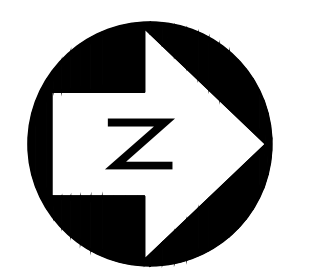
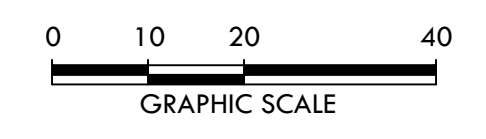
PROPOSED LAYOUT LEGEND

- PROPOSED BUILDING
- CONCRETE PAVEMENT
- CONCRETE SIDEWALK
- PROPERTY LINE
- EDGE OF NEW PAVEMENT

VEGETATION LEGEND

- TREES THAT WERE PLANTED WITH CONSTRUCTION OF THE HOUSE X 20
- NOTABLE NEW GROWTH X 15
- SAVED DURING CONSTRUCTION OF THE HOUSE
- PROPOSED TREES
- PROPOSED SHRUB

* NOTABLE NEW GROWTH DUE TO EXCESS SUNLIGHT ON THE SLOPE
* OTHER LARGE EXISTING TREES THROUGHOUT THE SLOPE NOT CURRENTLY SHOWN ON PLAN



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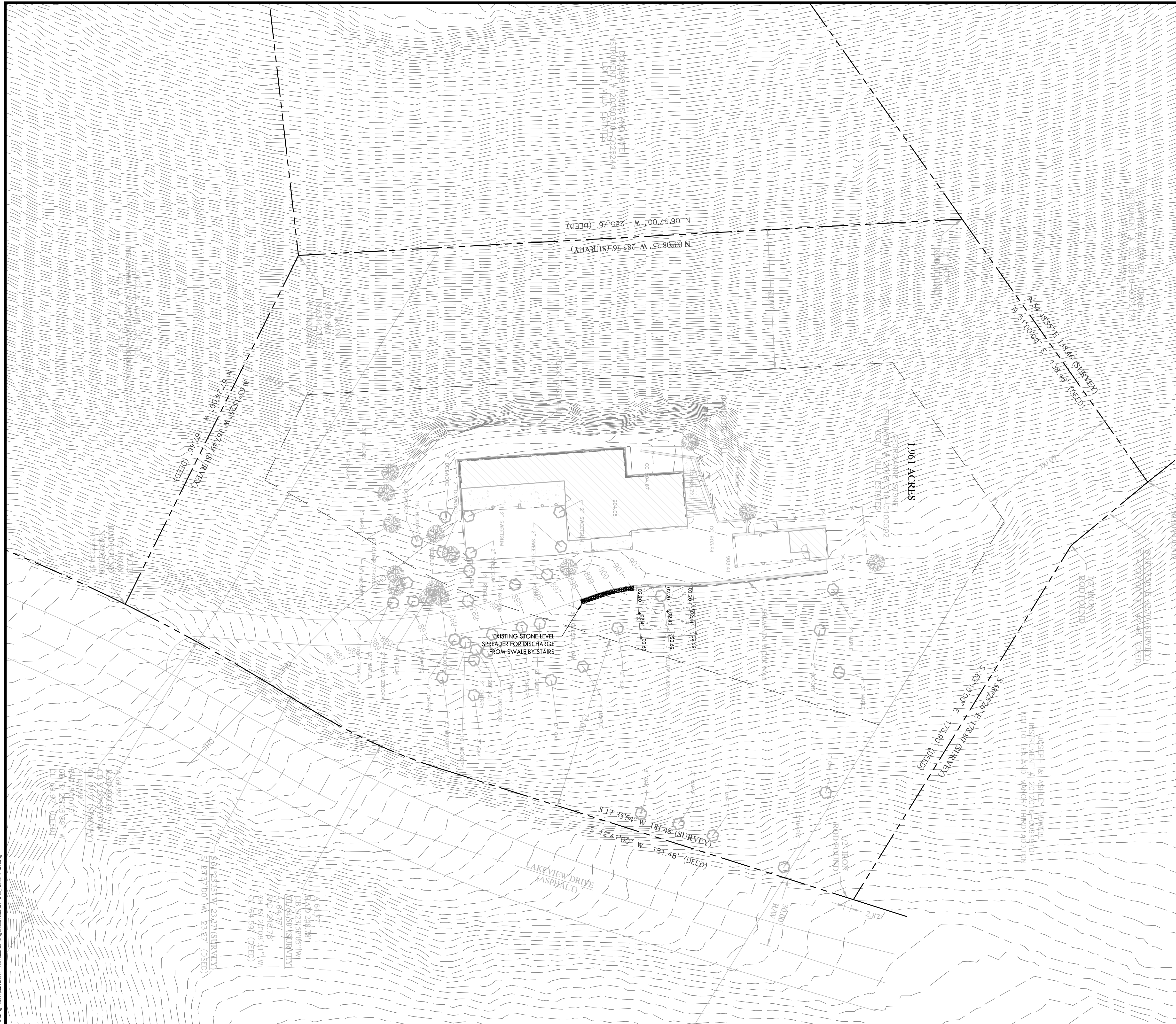
5031 LAKEVIEW DRIVE
NASHVILLE, TN

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10/26/23 21014

C3.0
LAYOUT PLAN

Drawn by: P. 10/26/2023 10:14:55 AM 5031 Lakeview Drive\5031 Lakeview Drive.dwg



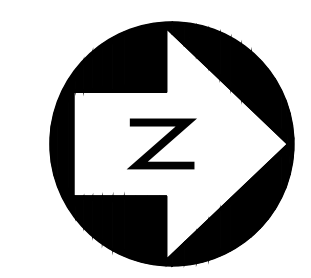
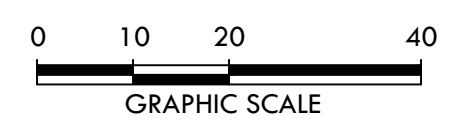
GRADING & DRAINAGE PLAN KEYNOTES

CODE	DESCRIPTION	DETAIL LOCATION
(G1)		

PROPOSED GRADING & DRAINAGE LEGEND

- PROPOSED CONTOUR
- PROPOSED SPOT ELEVATION
- STORM PIPE

NOTE: GRADING SURROUNDING PARKING PAD TO REMAIN CONSTANT



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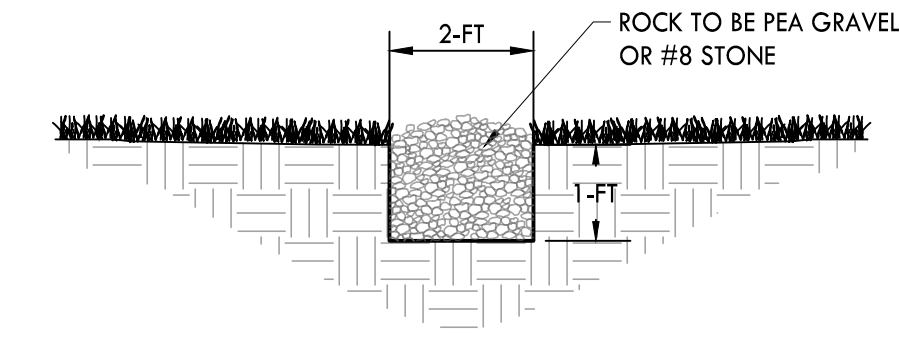
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No. 1	07/26/2023

08/22/23 21014

C4.0
 GRADING PLAN

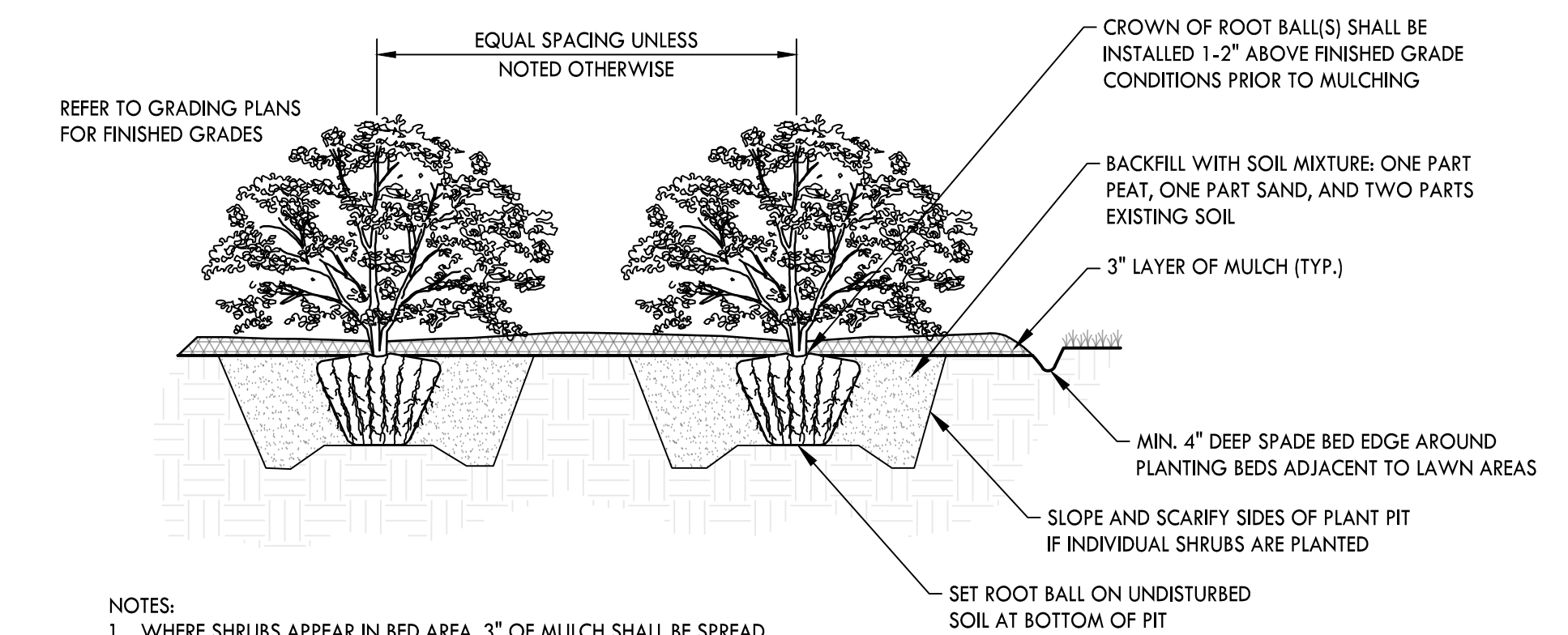
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1

LEVEL SPREADER
NOT TO SCALE

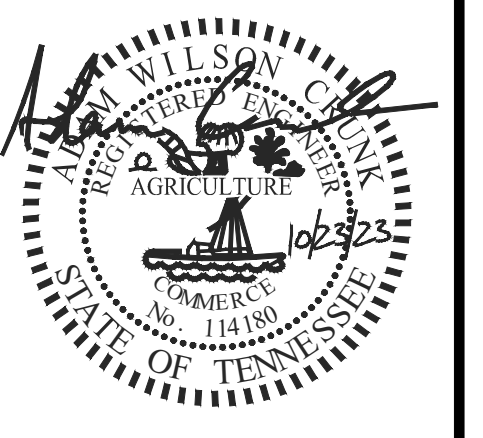


- NOTES:
1. WHERE SHRUBS APPEAR IN BED AREA, 3" OF MULCH SHALL BE SPREAD OVER THE ENTIRE AREA WITHIN THE BED AS INDICATED ON THE PLANS.
 2. DO NOT PRUNE SHRUBS BEFORE ACCEPTANCE.
 3. CAREFULLY REMOVE CONTAINER FROM ROOT BALL USED FOR SHRUB(S). ANY BROKEN OR CRUMBLING ROOT BALL(S) WILL BE REJECTED.
 4. IF SHRUB IS B&B, REMOVE BURLAP FROM TOP 1/2 OF ROOT BALL OR COMPLETELY REMOVE BURLAP IF SOILS ARE POORLY DRAINED.
 5. SCARIFY ROOTBALL IF ROOT BOUND IN CONTAINER PER HORTICULTURE STANDARDS.

2

SHRUB PLANTING
NOT TO SCALE

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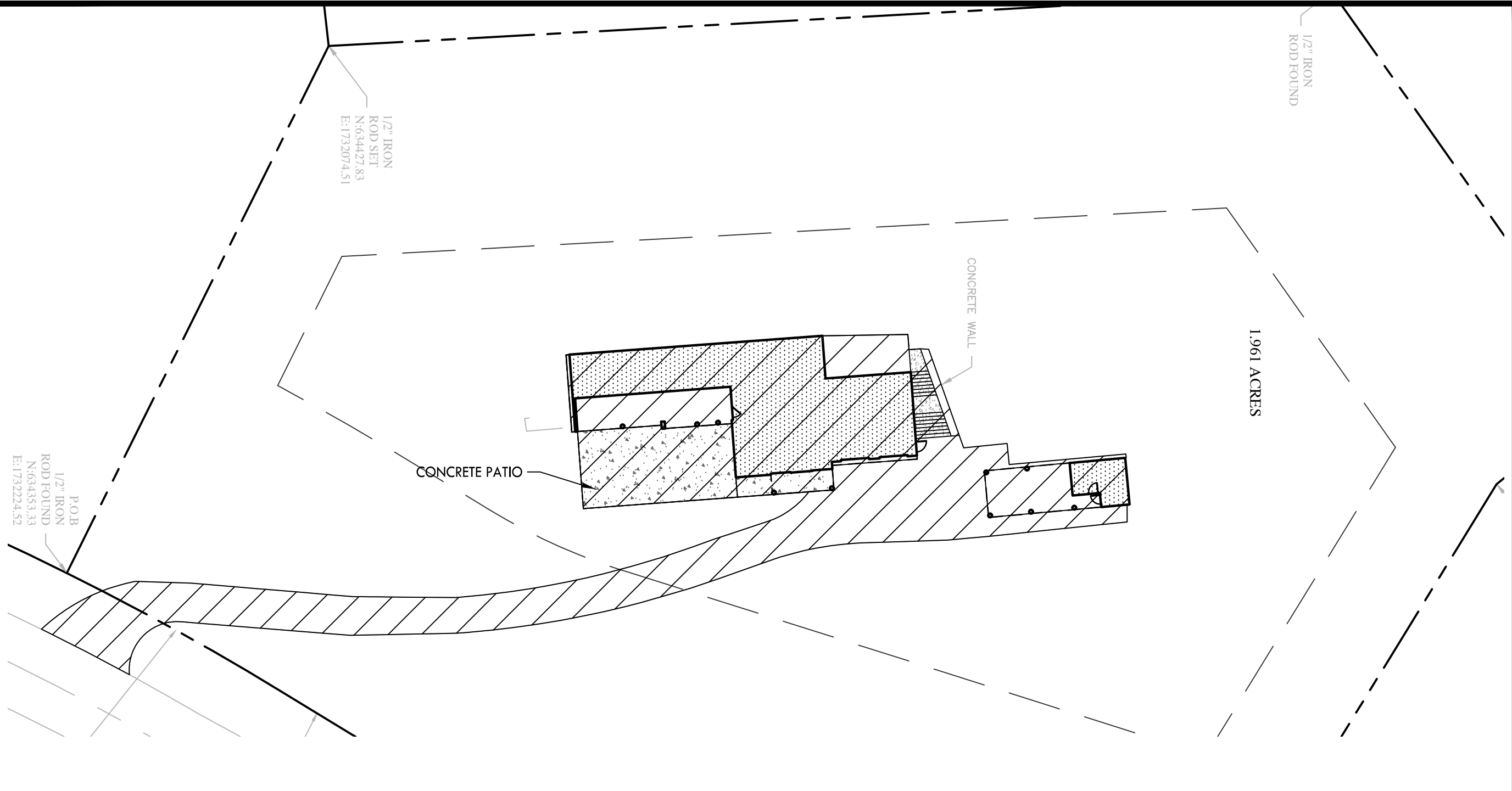
NASHVILLE, TN

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No.	DATE
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2	10/23/2023

10/23/23 21014

C5.0
DETAILS

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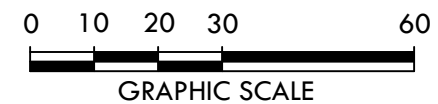


PERMITTED TOTAL IMPERVIOUS AREA = 8928.18 SQ FT

THIS IMPERVIOUS AREA WAS ORIGINALLY PERMITTED FOR CONSTRUCTION OF THE BUILDING AND ACCOMPANYING APPURTENANCES

LEGEND

 IMPERVIOUS AREA



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IMPERVIOUS AREA EXHIBIT

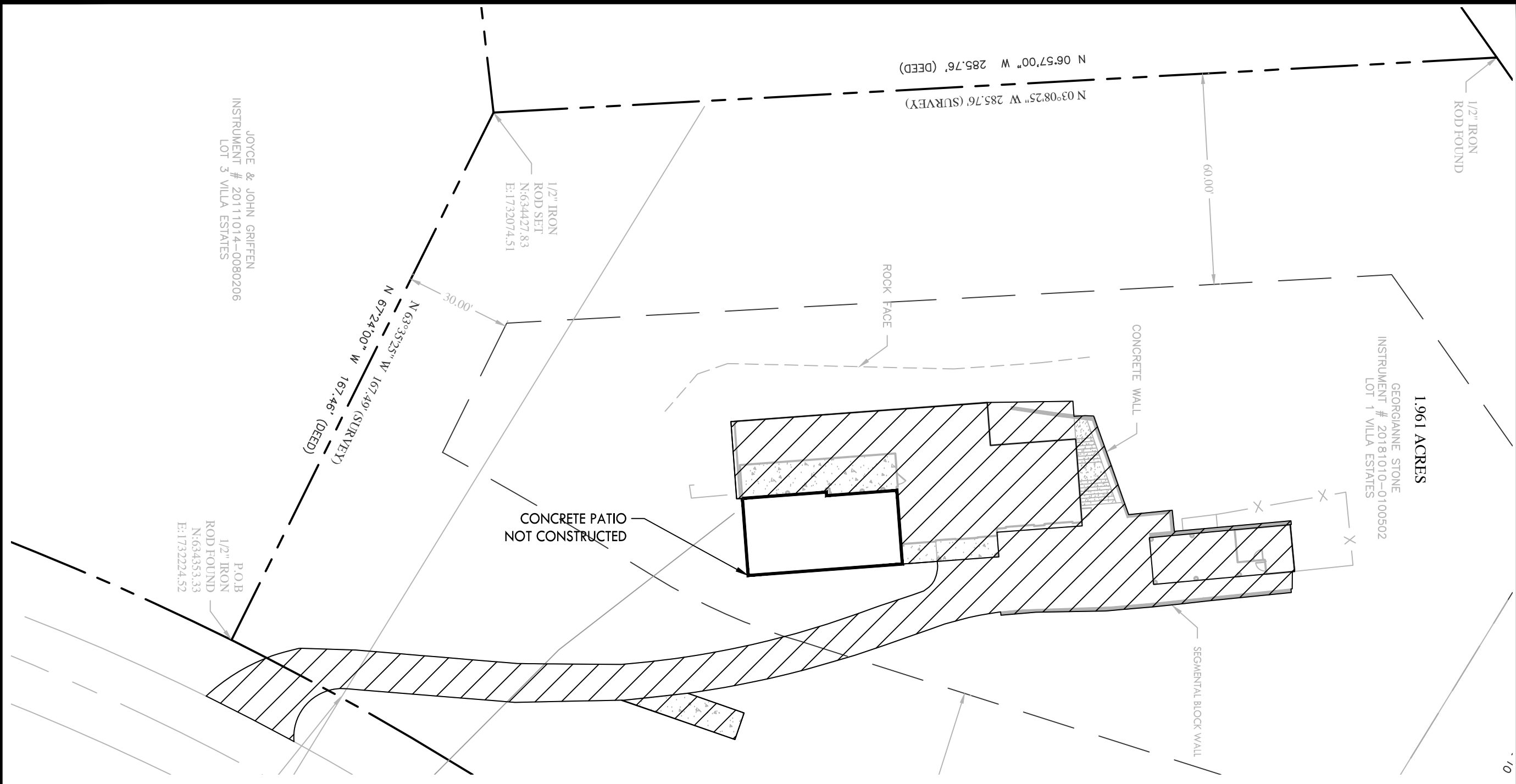
5031 LAKEVIEW DR

08/11/23 21014

EXHIBIT 1

ORIGINAL PROPOSED
IA

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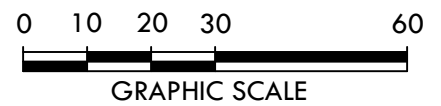


AS BUILT: TOTAL IMPERVIOUS AREA = 8185.55 SQ FT

THE CONCRETE PATIO WAS NOT CONSTRUCTED RESULTING IN A SIGNIFICANT REDUCTION IN TOTAL IMPERVIOUS AREA FROM WHAT WAS ORIGINALLY PERMITTED FOR CONSTRUCTION.

LEGEND

 IMPERVIOUS AREA



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IMPERVIOUS AREA EXHIBIT

5031 LAKEVIEW DR

08/11/23 21014

EXHIBIT 2

AS BUILT IA

JOYCE & JOHN GRIFFEN
 INSTRUMENT # 20111014-0080206
 LOT 3 VILLA ESTATES

P.O.B
 1/2" IRON
 ROD FOUND
 N:634353.33
 E:173224.52

1/2" IRON
 ROD SET
 N:634427.83
 E:1732074.51

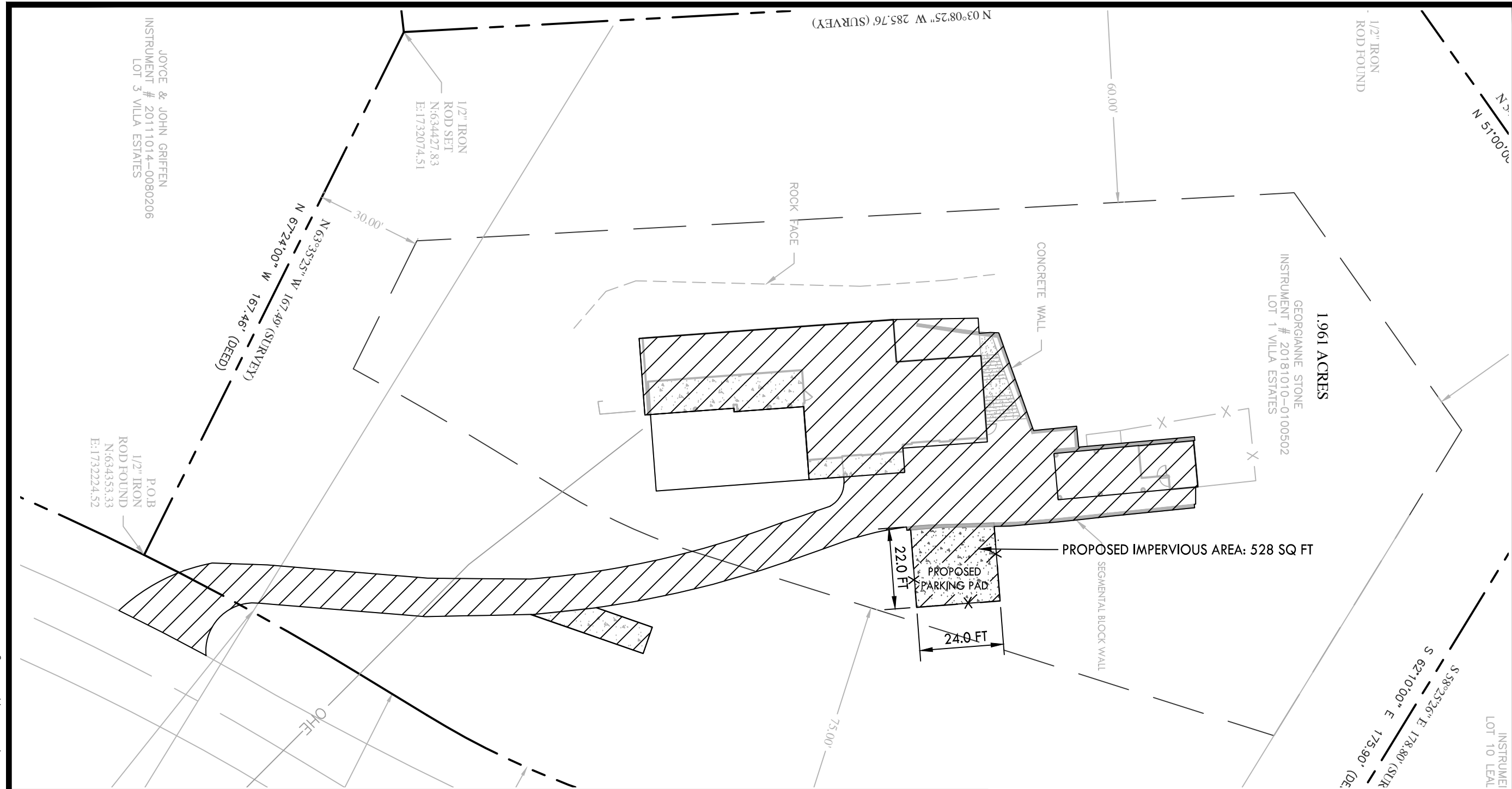
1.961 ACRES
 GEORGIANNE STONE
 INSTRUMENT # 20181010-0100502
 LOT 1 VILLA ESTATES

1/2" IRON
 ROD FOUND

N 03°08'25" W 285.76' (SURVEY)
 N 06°57'00" W 285.76' (DEED)

N 63°33'25" W 167.49' (SURVEY)
 N 67°24'00" W 167.46' (DEED)

Drawing Path: P:\2021\21014 - 5031 Lakeview\Drawg\Sketch\2023-08-01_Appendices\Appendix.dwg

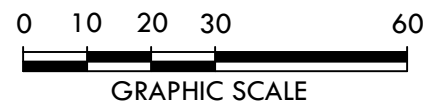


TOTAL IMPERVIOUS AREA INCLUDING PROPOSED PARKING PAD = 8713.55 SQ FT
 ORIGINAL PERMITTED TOTAL IMPERVIOUS AREA - PROPOSED TOTAL IMPERVIOUS AREA = 214.63 SQ FT
 8928.18 SQ FT - 8713.55 SQ FT = 214.63 SQ FT

THE ADDITION OF THE PROPOSED PARKING PAD WOULD ADD 528 SQ FT TO THE TOTAL AS-BUILT IMPERVIOUS AREA (8185.55 SQ FT) FOR A TOTAL IMPERVIOUS AREA OF 8713.55 SQ FT. THIS VALUE IS LESS THAN THE ORIGINALLY PERMITTED IMPERVIOUS AREA OF 8928.18 SQ FT

LEGEND

 IMPERVIOUS AREA



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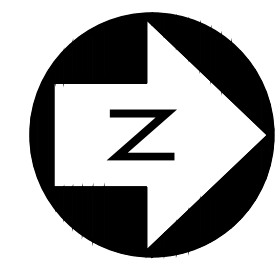
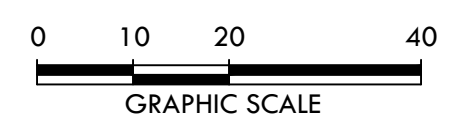
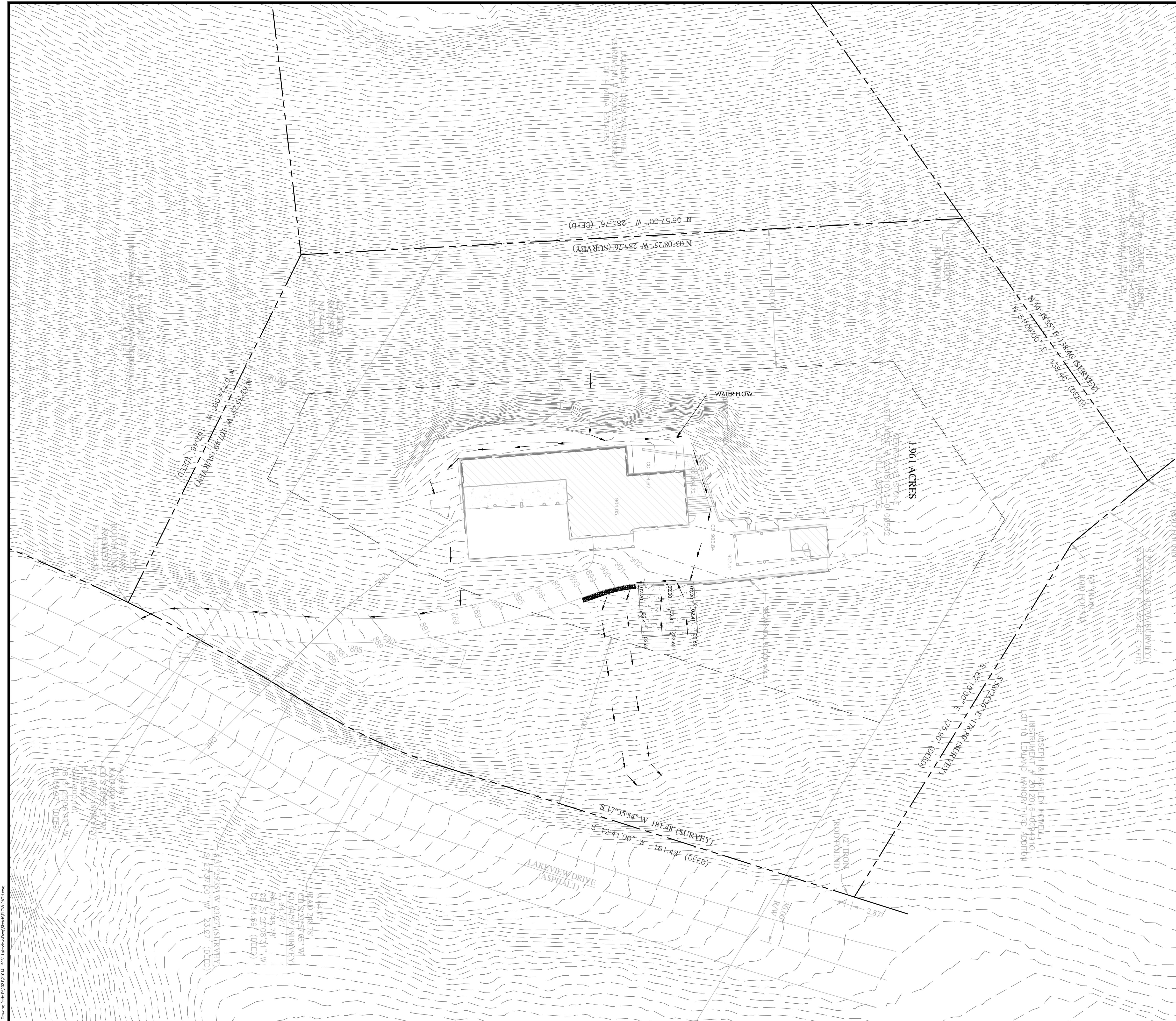
IMPERVIOUS AREA EXHIBIT

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EXHIBIT 3

PROPOSED PARKING PAD IA



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WATER FLOW CHART

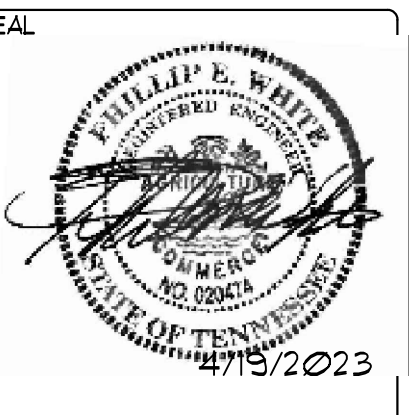
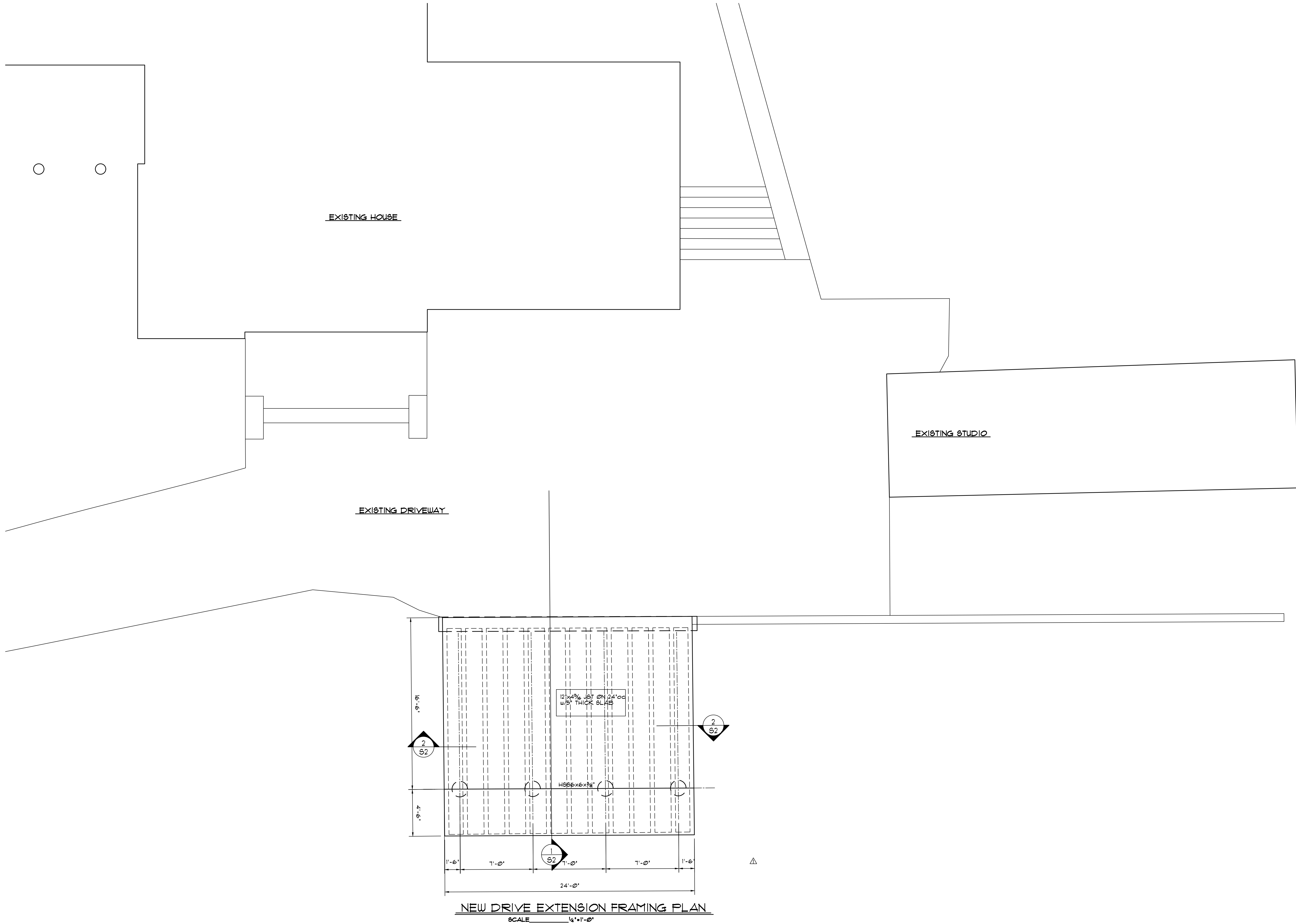
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REVISIONS

No.	DATE

08/03/23 21014

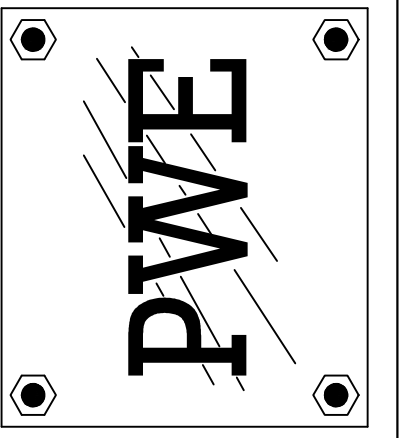
FLOW CHART



SCALE	NOTED
DRAWN BY	FEW
CHECKED BY	FEW
DATE	4/19/2023
APPROVED BY	FEW
JOB NO.	23026

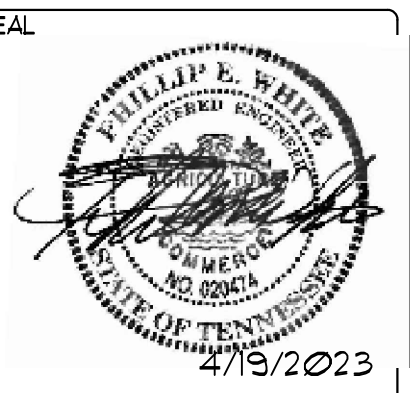
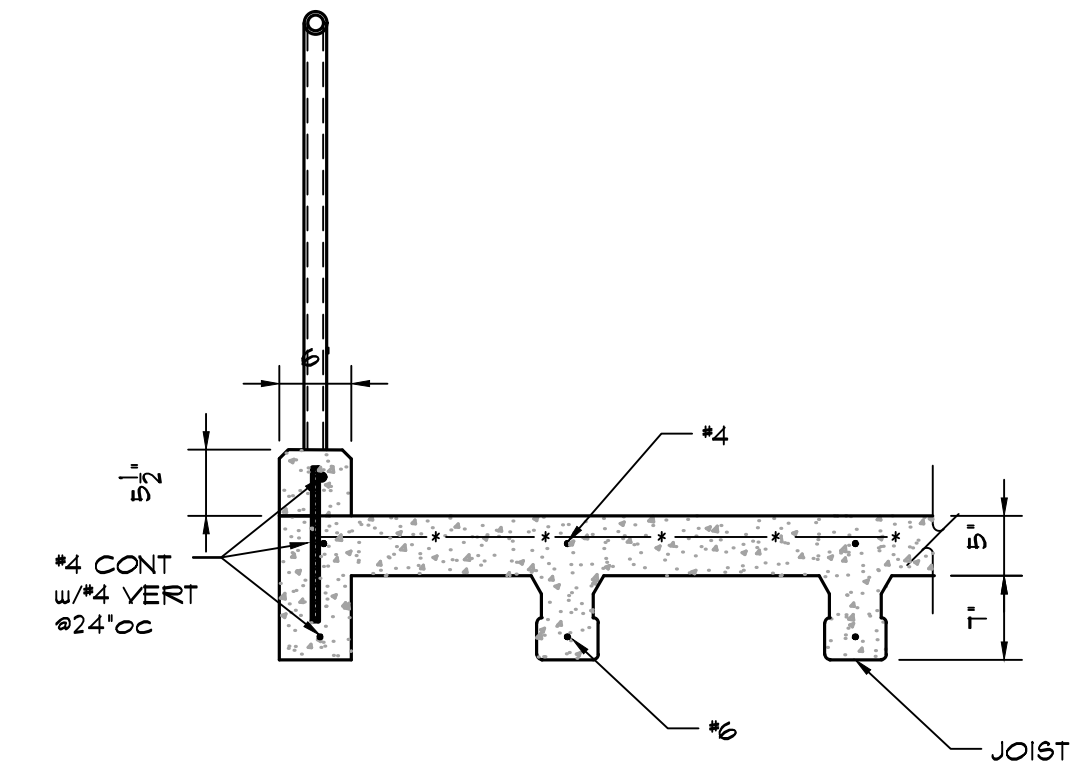
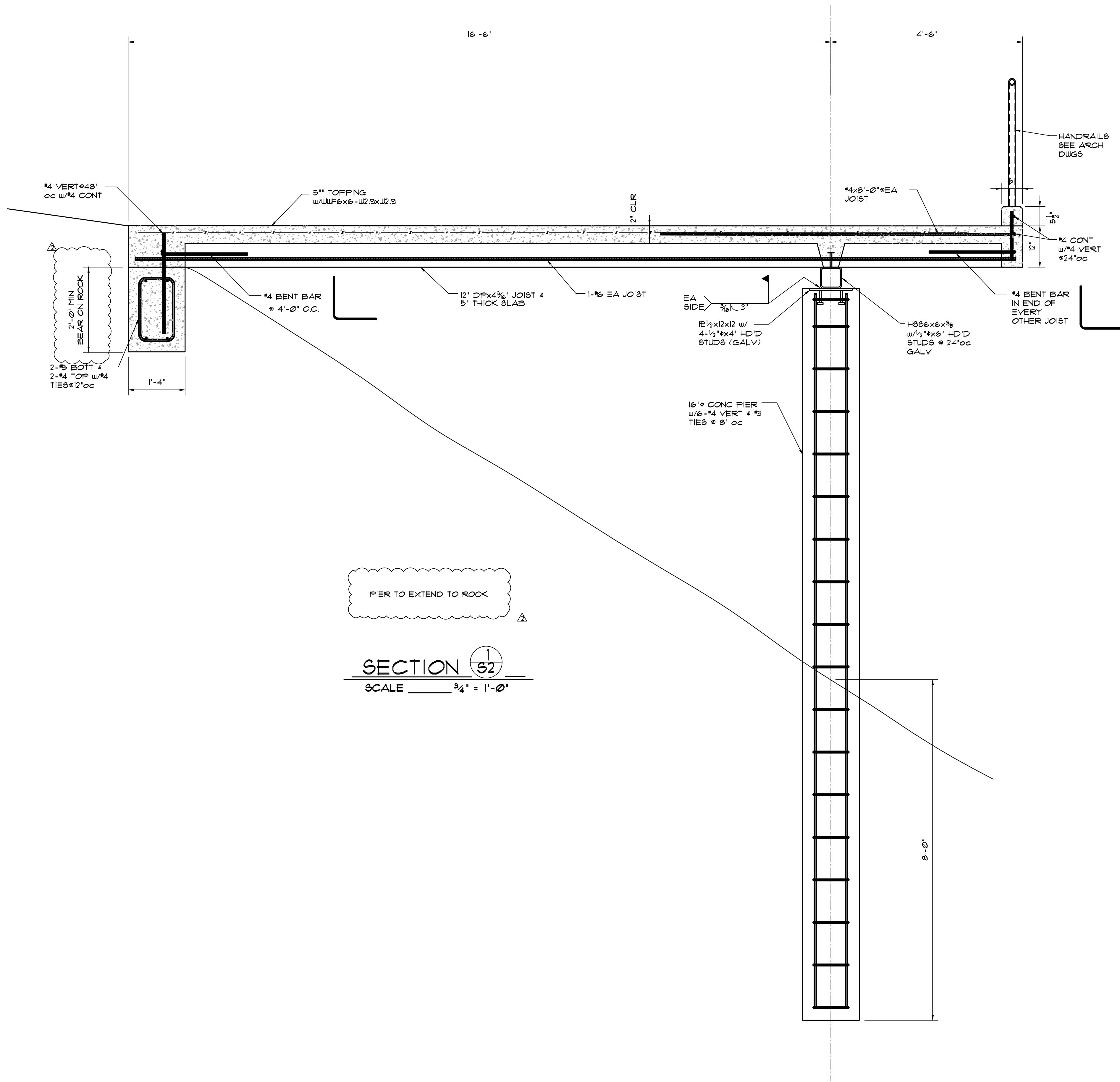
JOB TITLE DRIVEWAY EXTENSION
 5031 LAKEVIEW DRIVE
 NASHVILLE, TN
 SHEET TITLE
 FRAMING PLANS

PHILLIP
 WHITE
 ENGINEER
 912 HIDDEN OAK PL, BRENTWOOD, TN 37027
 PHONE: (615) 960-3506



NO.	DATE	REVISION
1	5/05/2023	CHANGE OF STRUCTURAL SYSTEM
2	5/03/2023	FOUNDATION NOTES

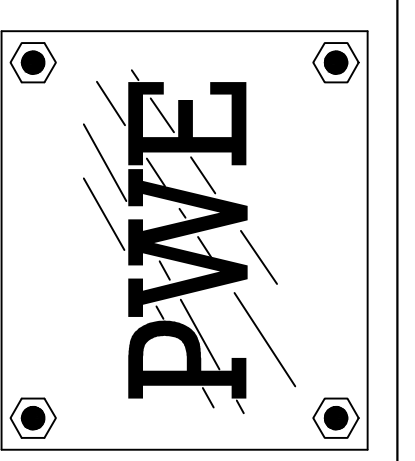
SHEET NO.
 S-1



SCALE	NOTED
DRAWN BY	FEW
CHECKED BY	FEW
DATE	4/19/2023
APPROVED BY	FEW
JOB NO.	23026

JOB TITLE DRIVEWAY EXTENSION
5031 LAKEVIEW DRIVE
NASHVILLE, TN
SECTION TITLE SECTIONS AND DETAILS

PHILLIP
WHITE
ENGINEER
912 HIDDEN OAK PL., BRENTWOOD, TN 37027
PHONE: (615) 960-3506



NO.	DATE	REVISION
1	5/05/2023	CHANGE OF STRUCTURAL SYSTEM
2	5/03/2023	FOUNDATION NOTES

GENERAL NOTES

DESIGN CRITERIA INFORMATION

- ALL CONSTRUCTION SHALL CONFORM TO THE 2018 EDITION OF THE INTERNATIONAL RESIDENTIAL CODE.
 - REPRODUCTION OF CONTRACT DOCUMENTS FOR USE AS SHOP DRAWINGS IS NOT PERMITTED.
 - CONTRACTOR SHALL VERIFY EXISTING CONDITIONS AND DIMENSIONS AND NOTIFY ARCHITECT OF ANY CONDITIONS WHICH DO NOT COMPLY WITH PLANS AND SPECIFICATIONS.
 - SERVICE DESIGN LIVE LOADS ARE AS FOLLOWS:
PARKING AREA: 50 PSF
- HANDRAILS: (APPLIED AT ANY POINT IN ANY DIRECTION)
1. 200# CONCENTRATED AT TOP RAIL
2. 50 PLF LOADS 1 & 2 ARE NOT APPLIED SIMULTANEOUSLY
- GUARDRAILS: (APPLIED TO TOP RAIL)
1. 200# CONCENTRATED
2. 50 PLF HORIZONTAL LOAD W/ 100# PLF VERTICAL LOAD LOADS 1 & 2 ARE NOT APPLIED SIMULTANEOUSLY
- THE DESIGN SNOW LOAD CRITERIA IS AS FOLLOWS:
FLAT ROOF SNOW LOAD P_f 5 PSF
SNOW EXPOSURE FACTOR C_e 1.0
SNOW IMPORTANCE FACTOR I_s 1.0
THERMAL FACTOR C_t 1.0
- THE WIND DESIGN CRITERIA IS AS FOLLOWS:
ULTIMATE DESIGN WIND SPEED 115 MPH
WIND IMPORTANCE FACTOR 1.0
EXPOSURE 3
BASIC WIND PRESSURE 24.0 PSF
GCPI 1.0

FOUNDATION NOTES

- FOOTINGS FOR THE ABUTMENT WERE DESIGNED FOR AN ASSUMED ASSUMED ALLOWABLE BEARING PRESSURE OF 15000 PSF.
- THE BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BEAR ON ROCK PER THE GEOTECHNICAL REPORT. SHALL BE MINIMUM OF 1'-6" BELOW FINISHED GRADE
- WHERE FOOTINGS STEPS ARE NECESSARY, THEY SHALL BE NO STEEPER THAN 1 VERTICAL TO 2 HORIZONTAL EXCEPT AS SHOWN ON THE CONTRACT DOCUMENTS.

MISCELLANEOUS

- FOR LOCATION OF THE MISCELLANEOUS ITEMS (SUCH AS OPENINGS, INSERTS, ETC.) AFFECTING STRUCTURAL WORK, SEE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS. ALL OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER.
- ALL CAVITY WALLS SHALL HAVE 3/16" DIAMETER CAVITY WALL TIES SPACED TO PROVIDE AT LEAST ONE TIE FOR EACH 3 SQUARE FEET OF WALL.
- USE LIGHTWEIGHT BLOCK PARTITIONS ON ALL FRAMED FLOORS.
- WHERE LINTELS BEAR ON MASONRY WALLS, THEY SHALL BEAR ON EITHER A BOND BEAM COURSE OR ON CORES FILLED WITH CONCRETE. ALL LINTELS SHALL HAVE AT LEAST 8" OF BEARING AT EACH END UNLESS NOTED OTHERWISE.
- ALL CONCRETE WEDGE-TYPE ANCHORS SHOWN SHALL BE HILTI WEDGE ANCHORS OR EQUAL.

REINFORCED CONCRETE

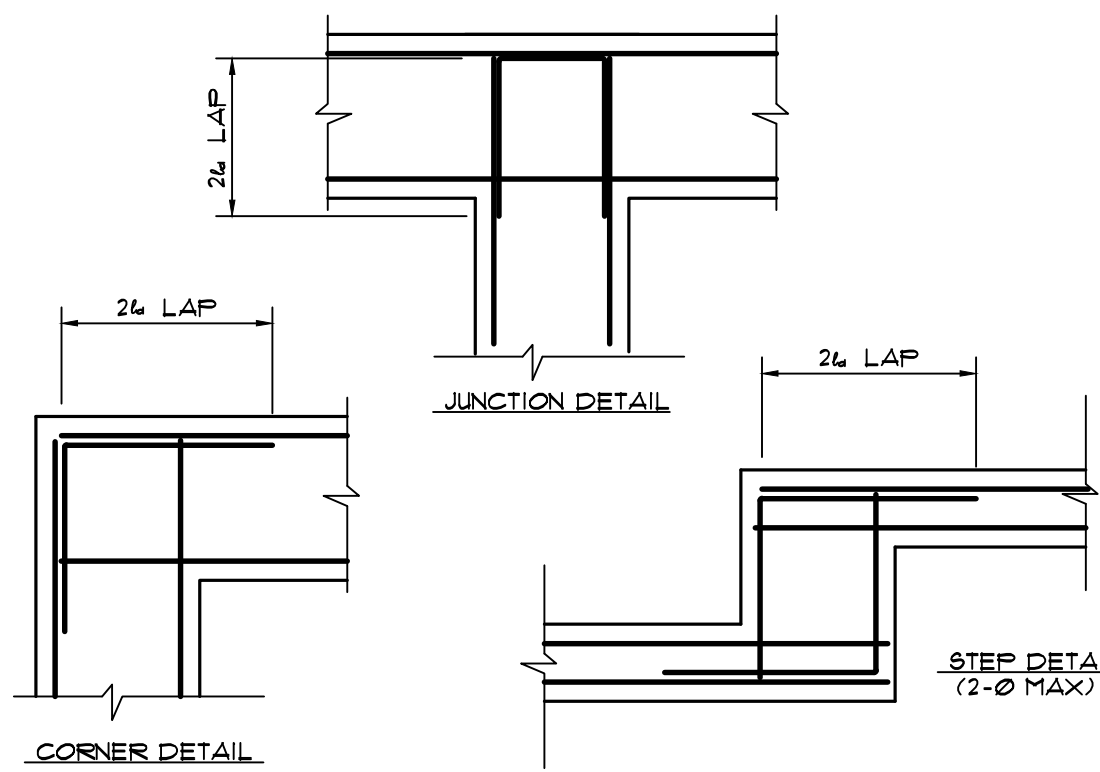
- ALL CONCRETE WORK SHALL CONFORM TO ACI 318, 'BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE'.
- THE 28-DAY STRENGTH OF CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:
SLAB-ON-GRADE 3000 PSI
FOOTINGS 3000 PSI
- THE USE OF FLY ASH IS PERMITTED. CONCRETE MIX DESIGNS SHALL BE SUBMITTED AND APPROVED PRIOR TO CASTING OF ANY CONCRETE.
- ALL CONCRETE PLACED SHALL BE VIBRATED BY MECHANICAL VIBRATORS.
- REINFORCING BARS SHALL CONFORM TO ASTM A615, 'SPECIFICATION FOR DEFORMED AND PLAIN BILLET-STEEL BARS FOR CONCRETE REINFORCEMENT'. THE MINIMUM YIELD STRESS OF REINFORCING BARS SHALL BE 60,000 PSI.
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 'SPECIFICATION FOR WELDED STEEL WIRE FABRIC FOR CONCRETE REINFORCEMENT'.
- COMPLETE DRAWINGS FOR FABRICATION AND PLACING OF REINFORCING STEEL SHALL BE SUBMITTED FOR APPROVAL. NO FABRICATION MAY BEGIN UNTIL DRAWINGS ARE COMPLETED AND APPROVED.
- LAP SPICES FOR REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ACI 318. ALL UNSPECIFIED LAP SPICES SHALL BE MAXIMUM LENGTH.
- REINFORCING OF ALL CONCRETE MEMBERS SHALL HAVE THE FOLLOWING CLEAR CONCRETE COVER:
COVER INCHES:
CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3
CONCRETE EXPOSED TO EARTH OR WEATHER:
#6 THROUGH #8 BARS 2
#9 BAR, W31 OR D31 WIRE, AND SMALLER 1-1/2
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
SLABS, WALLS, JOISTS: #4 AND #6 BARS 1-1/2
#1 BAR AND SMALLER 3/4
BEAMS, COLUMNS: PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS 1-1/2
SHELLS, FOLDED PLATE MEMBERS: #6 BAR AND LARGER 3/4
#9 BAR, W31 OR D31 WIRE, AND SMALLER 1/2
- THE CONTRACTOR SHALL PROVIDE CHAIRS AT 36 INCH CENTER TO CENTER TO SUPPORT WIRE MESH WHILE CASTING SLABS. FULL FABRIC UP BETWEEN SUPPORTS TO PROVIDE 2" CLEARANCE TO TOP OF SLAB. MINIMUM SIDE AND END LAP ON FABRIC SHALL BE ONE WIRE SPACE.
- ANCHOR BOLTS SHALL BE F1554 GR 36 WITH HIGH STRENGTH NUTS.
- WELDING OF REINFORCING STEEL SHALL BE DONE IN STRICT ACCORDANCE WITH THE AMERICAN WELDING SOCIETY 'REINFORCING STEEL WELDING CODE', AWS D1.4-18. PREHEATING OF REINFORCING SHALL BE BASED ON THE CARBON EQUIVALENT DETERMINED FROM REINFORCING MILL REPORTS. GRADE 40 REINFORCING SHALL BE WELDED WITH E70XX LOW HYDROGEN ELECTRODES, AND GRADE 60 REINFORCING SHALL BE WELDED WITH E90XX LOW HYDROGEN ELECTRODES.

PRECAST/PRESTRESSED CONCRETE UNITS

- ALL PRECAST/PRESTRESSED CONCRETE UNITS SHALL BE DESIGNED AND MANUFACTURED IN ACCORDANCE WITH ACI 318, BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE.
- ALL PRECAST/PRESTRESSED CONCRETE UNITS SHALL BE PRODUCED IN ACCORDANCE WITH PNL-116, 'MANUAL FOR QUALITY CONTROL FOR PLANTS AND PRODUCTION OF PRECAST/PRESTRESSED CONCRETE PRODUCTS' PUBLISHED BY THE PRESTRESSED CONCRETE INSTITUTE.
- THE STRENGTH OF ALL PRECAST CONCRETE SHALL BE 5000 PSI AT 28 DAYS. THE 28-DAY STRENGTH OF PRECAST COLUMNS SHALL BE 6000 PSI. THE STRENGTH AT STRIPPING OR RELEASE OF PRESTRESSING STRAND SHALL BE 3000 PSI, UNLESS NOTED.
- PRESTRESSING STRAND FOR ALL PRESTRESSED MEMBERS SHALL BE SEVEN WIRE STRANDS MADE OF GOLD-DRAIN STRESS-RELIEVED OR LOW-RELAXATION WIRE CONFORMING TO ASTM A-416 WITH A MINIMUM ULTIMATE STRENGTH OF 210,000 PSI UNLESS NOTED OTHERWISE.
- HOLLOWCORE SLABS SHALL BE GROUTED TOGETHER FOLLOWING THE ERECTION OF EACH 30000 SQUARE FEET INCREMENT. DIFFERENTIAL CAMBER BETWEEN SLABS SHALL BE ELIMINATED BY APPLICATION OF DEAD WEIGHT OR MINIMAL JACKING BEFORE GROUTING IS DONE. CORRECTIVE CAMBER LOAD SHALL BE HELD ON UNTIL GROUT HAS CURED.
- GROUT SHALL FILL JOINT AT ENDS OF ADJACENT PLANK OVER BEARING WALLS/PRECAST/PRESTRESSED CONCRETE UNITS

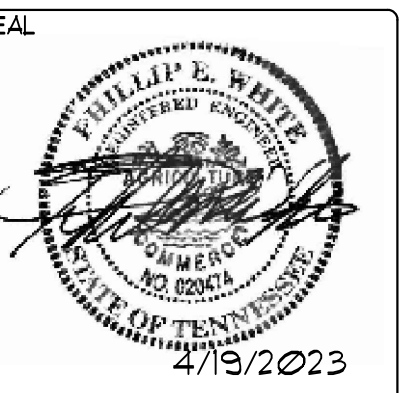
STRUCTURAL STEEL

- ALL STRUCTURAL STEEL WORK SHALL CONFORM TO THE 'SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS' AISC 360-10
- SHOP DRAWINGS FOR ALL STRUCTURAL STEEL SHALL BE SUBMITTED AND APPROVED PRIOR TO ANY FABRICATION.
- STRUCTURAL STEEL SHALL MEET THE FOLLOWING ASTM SPECIFICATIONS:
• STRUCTURAL PIPE: A53 TYPE E OR S, GRADE B, WITH SULPHUR CONTENT NOT TO EXCEED .06%.
• STRUCTURAL TUBE: A500 GRADE B.
• STEEL JOISTS: AS REQUIRED BY THE STANDARD SPECIFICATIONS OF THE STEEL JOIST INSTITUTE.
• W-SHAPES: A992 UNLESS NOTED OTHERWISE ON CONTRACT DOCUMENTS.
• ALL OTHER FRAMING: A36 UNLESS NOTED OTHERWISE ON CONTRACT DOCUMENTS.
- STEEL FRAMING CONNECTIONS SHALL BE BOLTED OR WELDED. BOLTS SHALL BE A MINIMUM OF 3/4" DIAMETER AND SHALL BE ASTM A325. EACH FASTENER SHALL BE TIGHTENED TO THE MINIMUM TENSION FOR THE SIZE AND GRADE OF FASTENER USED AS DETERMINED BY ONE OF THE FOLLOWING METHODS:
• TURN OF THE NUT (A.I.S.C.)
• CALIBRATED WRENCH
• LOAD INDICATOR WASHERS
• LOAD INDICATOR BOLTS.
- BEAMS SHALL BE FABRICATED AND ERECTED WITH NATURAL CAMBER UP.
- ALL WELDS SHALL CONFORM TO AWS D1.1, 'STRUCTURAL WELDING CODE'. ALL GROOVE WELDS SHOWN ON CONTRACT DOCUMENTS SHALL BE FULL PENETRATION UNLESS NOTED OTHERWISE. WELDING SHALL BE DONE WITH E-7018 ELECTRODES UNLESS NOTED OTHERWISE.
- () DENOTES DEVIATION FROM TOP OF STEEL ELEVATION IN INCHES.
- STRUCTURAL STEEL EMBEDDED IN CONCRETE SHALL NOT BE PAINTED.
- GROUT USED IN GROUT BEDS UNDER COLUMNS BASE PLATES SHALL BE CEMENT BASED, NON-SHRINK GROUT. THE GROUT SHALL EXHIBIT NO SHRINKAGE IN ACCORDANCE WITH ASTM C827, 'TEST METHOD FOR EARLY VOLUME CHANGE OF CEMENTITIOUS MIXTURES' AND SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 5000 PSI WHEN TESTED IN ACCORDANCE WITH ASTM C-109, 'TEST METHOD FOR COMPRESSIVE STRENGTH OF HYDRAULIC CEMENT MORTARS.'
- STRUCTURAL STEEL FRAMING SHALL BE ERECTED TRUE AND PLUMB IN ACCORDANCE WITH A.I.S.C. CODE OF STANDARD PRACTICE. ANY FRAMING EXCEEDING TOLERANCES OF THE CODE OF STANDARD PRACTICE SHALL BE CORRECTED BY THE CONTRACTOR AT HIS EXPENSE AS DIRECTED BY THE STRUCTURAL ENGINEER.
- THE STRUCTURAL STEEL ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STRUCTURAL STEEL FRAME WORK AGAINST LATERAL LOADINGS SUCH AS WIND. THIS BRACING SHALL REMAIN IN PLACE UNTIL THE FINAL SYSTEM FOR RESISTING LATERAL LOADS IS IN PLACE AND EFFECTIVE AS APPROVED BY THE STRUCTURAL ENGINEER.
- STEEL COLUMNS EXTENDING BELOW GRADE AND NOT ENCASED WITH CONCRETE SHALL BE COATED WITH FITCH.
- SHEAR CONNECTIONS FOR NON-COMPOSITE BEAMS SHALL BE DESIGNED FOR THE LOAD CAPACITY OF A SIMPLE SPAN FULLY SUPPORTED BEAM OR THE REACTION SHOWN ON THE PLAN, WHICHEVER IS GREATER.



TYPICAL FOOTING & WALL DETAILS

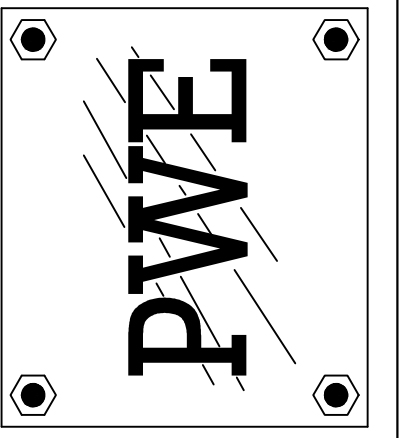
NO SCALE



SCALE	NOTED
DATE	4/19/2023
JOB NO.	23026
DRAWN BY	FEW
CHECKED BY	FEW
APPROVED BY	FEW

JOB TITLE: DRIVEWAY EXTENSION
5031 LAKEVIEW DRIVE
NASHVILLE, TN
SHEET TITLE: GENERAL NOTES

PHILLIP WHITE
ENGINEER
912 HIDDEN OAK PL, BRENTWOOD, TN 37027
PHONE: (615) 960-3506



NO.	DATE	REVISION
1	5/05/2023	CHANGE OF STRUCTURAL SYSTEM
2	5/13/2023	FOUNDATION NOTES

SHEET NO. S-3

Charles D. Brown
1043 Ridgeview Drive
Nashville, TN 37220

October 3, 2023

City of Oak Hill
Planning Commission
PC Case 23-22

To the members of the City of Oak Hill Planning Commission:

I am unable to attend tonight's planning commission meeting due to a previous commitment. However, I note that I have expressed our concerns about the steep slope, soils, and the drainage on the properties above my house at 1043 Ridgeview Drive. These concerns have not changed. I have also questioned past actions and commitments made in addressing these concerns. Hopefully, any potential changes on the steep slope will take these into consideration.

In addition, we are taking additional action on our personal property to help minimize future problems. Our actions are currently being permitted through the City of Oak Hill.

We support the full review of this case by the Planning Commission and will support any outcome. Thank you to the City of Oak Hill for the time and thoughtful efforts in serving our community.

Sincerely,



Charles D. Brown